

PERMIT

CITY OF NAPOLEON - BUILDING DEPARTMENT

255 West Riverview Avenue, Napoleon, Ohio 43545 - 419-592-4010

Permit No. 01901 Issued 1-5-90
date

Job Location 960 Harmony Drive
address

Lot 34 & PT35 Brickyard
sub-div or legal discript

Issued By Eldon Huber
building official

Owner Joe Celani 599-3464
name tel.

Address 1140 E. Riverview Apt. 1D

Agent Paul Celani 337-3902
builder-eng.-etc. tel.

Address 429 W. Elm Wauseon

Description of Use Residence

Residential 1
no. dwelling units

Commercial _____ Industrial _____

New Add'n. _____ Alter _____ Remodel _____

Mixed Occupancy _____

Change of Occupancy _____

Estimated Cost \$ 65000.00

ZONING INFORMATION

district	lot dimensions		area	front yd	side yds		rear yd
A	94'	X 128.01'	12,033 SF	30'	13'4"L	19'R	35'
max hgt	no pkg spaces		no ldg spaces	max cover	petition or appeal req'd		date appr
35'	2-min			35%			

WORK INFORMATION:

Size: Length 60'8" Width 62'8" Stories 1 Garage Fl. Area 598
Height 13' Building Volume (for demo. permit) _____ cu. ft.
1st Floor Area 1600
Ground Floor Area 1841

Electrical: New construction 30 new circuits
brief description

Plumbing: New construction
brief description

Mechanical: New H.V.A.C. system
brief description

Sign: _____ Dimensions _____ Sign Area _____
type

Additional Information: New residence

Date 2-20-90 Applicant Signature Paul J. Celani owner-agent

PAID
FEB 20 1990

FEES	BASE	PLUS	TOTAL
<input checked="" type="checkbox"/> BUILDING	9.00	158.00	167.00
<input checked="" type="checkbox"/> ELECTRICAL	15.00	90 90.00	105.00
<input checked="" type="checkbox"/> PLUMBING	9.00	36.00	45.00
<input checked="" type="checkbox"/> MECHANICAL	18.00	10.00	28.00
<input checked="" type="checkbox"/> DEMOLITION			
<input type="checkbox"/> ZONING	5.00	.00	5.00
<input checked="" type="checkbox"/> SIGN			
<input type="checkbox"/> WATER TAP	375.00		375.00
SEW. INSP.			
SEWER TAP	60.00	.00	60.00
TEMP. WATER	5.00		5.00
TEMP. ELECT.	10.00		10.00
ADDITIONAL PLAN REVIEW	Struct. _____ hrs	Elect. <u>ADD 1- FLOOR DR. 3.00</u> <u>1- CORK DOOR 20.00</u>	<u>3.00</u> <u>20.00</u>
TOTAL FEES			<u>1025.00</u>
LESS MIN. FEES PAID			<u>1025.00</u>
BALANCE DUE			_____

INSPECTION RECORD

	UNDERGROUND			ROUGH-IN						FINAL		
	Type	Date	By	Type	Date	By	Type	Date	By	Type	Date	By
PLUMBING	Building Drains			Drainage, Waste & Vent Piping			Indirect Waste			Drainage, Waste & Vent Piping		
	Water Piping									Backflow Prevention		
	Building Sewer			Water Piping			Condensate Lines			Water Heater		
	Sewer Connection									FINAL APPROVAL		
MECHANICAL	Refrigerant Piping			Refrigerant Piping			Chimney(s)			Grease Exhaust System		
				Duct Furnace(s)			Fire Dampers			Air Cond. Unit(s)		
	Ducts/ Plenums			Ducts/ Plenums	8/6	BD	<input type="checkbox"/> Radiant Htr(s) <input type="checkbox"/> Unit Htr(s)			Refrigeration Equipment		
				Duct Insulation			Pool Heater			Furnace(s)		
				Combustion Products Vents			Ventilation <input type="checkbox"/> Supply <input type="checkbox"/> Exhst.			FINAL APPROVAL		
ELECTRICAL	Conduits & or Cable			Conduits/ Cable			<input checked="" type="checkbox"/> Range <input checked="" type="checkbox"/> Dryer			Temp Service Temp Lighting		
	Grounding & or Bonding			Rough Wiring	8/6	BD	<input type="checkbox"/> Generator(s) <input type="checkbox"/> Motors			Fixtures Lampholders		
	Floor Ducts Raceways			Service Panel Switchboard	8/6	BD	<input type="checkbox"/> Water Htr <input type="checkbox"/> Welder			Signs		
	Service Conduit			Busways Ducts			<input type="checkbox"/> Heaters <input type="checkbox"/> Heat Cable			Electric Mtr. Clearance		
	Temporary Power Pole			Subpanels			<input type="checkbox"/> Duct Htr(s) <input type="checkbox"/> Furnace(s)			FINAL APPROVAL		
BUILDING	Location, Set-backs, Esmt(s)		EH	Exterior Wall Construction		EH	Roof Covering Roof Drainage	6/20	EH	Smoke Detector		
	Excavation		EH				Exterior Lath			Demolition (sewer cap)		
	Footings & Reinforcing		EH				<input type="checkbox"/> Interior Lath <input type="checkbox"/> Wallboard					
	Floor Slab			Interior Wall Construction		EH	Fire Wall(s)			Building or Structure		
	Foundation Walls		EH	Columns & Supports			Fireplace Chimney					
	Sub-soil Drain			Crawl Space <input type="checkbox"/> Vent <input checked="" type="checkbox"/> Access		EH	Attic <input type="checkbox"/> Vent <input type="checkbox"/> Access					
	Piles			Floor System(s)	*	EH				FINAL APPROVAL BLDG. DEPT.		
				Roof System		EH	Special Insp Reports Rec'd			Certificate of Occupancy Issued		
ADDITIONAL	INSPECTIONS, CORRECTIONS, ETC.						INSPECTIONS, CORRECTIONS, ETC.					
	* CHECK CATRAN FOR LOD BUC BTWN 10/15/21						6/20 EH					

APPLICATION
for
RESIDENTIAL BUILDING, ELECTRICAL, PLUMBING, MECHANICAL, PERMITS and DEMOLITION PERMIT
from the

CITY OF NAPOLEON - BUILDING DEPARTMENT

Entry No. _____

255 West Riverview Ave. Napoleon, Ohio 43545 Pn. 419-592-4010

Permit No. 01901 Issued 1-5-89

Job Location 960 Harmony Drive

Lot 34 + 1735 Berk yards
sub-div. or legal disc.

Issued By F building official

Owner Joe Celani Pn 599-3464

Address 1140 East Riverview Apt 1D

Agent PAUL CELANI Pn 377-3902

Address 429 W. ELK WADSWORTH OHIO

Description of Use RESIDENCE
new construction

Residential Single family 1
no. dwelling units

Commercial _____ Industrial _____

New Add'n. _____ Alter _____ Remodel _____

Mixed Occupancy _____

Change of Occupancy _____

Estimated Cost \$ 65000⁰⁰

-ZONING INFORMATION

district A lot dimensions 94' x 128.01' area 12,033 S.F. front yd 30' side yds. 13'-4" - 19'-2" rear yd 35'

max hgt 35' no pkg spaces 2-MIN no ldg spaces _____ max cover 35% petition or appeal req'd. _____ date appr _____

WORK INFORMATION:

BUILDING: Garage Fl. Area 598 sqft 12th floor Basement Fl. Area 1600 sqft Second Floor Area _____

Size: Length 60'-8" Width 62'-8" Stories 1 Ground Floor Area 1841

Height 13' Building Volume (for demo. permit) _____ cu. ft.

Description of Work: NEW RESIDENCE

Ck. Permits Reg.	Base	Fees Plus	Total
<input checked="" type="checkbox"/> Building	9.00	158 158 .00	167.00
<input checked="" type="checkbox"/> Electrical	15.00	90.00	225.00
<input checked="" type="checkbox"/> Plumbing	9.00	36.00	45.00
<input checked="" type="checkbox"/> Mechanical	18.00	10.00	28.00
Demolition			
<input checked="" type="checkbox"/> Zoning	5.00	1.00	6.00
Sign			
<input checked="" type="checkbox"/> Water tap	375.00	1.00	376.00
<input checked="" type="checkbox"/> Sewer Tap	60.00	1.00	61.00
<input checked="" type="checkbox"/> Temp. Water	5.00		5.00
<input checked="" type="checkbox"/> Temp. Elec.	10.00		10.00

Additional struc. _____ hrs
plan review _____
Elect. _____ hrs
Total Fees..... 1025.00
Less Min. Fees Pd. _____
date _____
Balance Due..... _____

ELECTRICAL: Electrical Contractor Joe Celani Pn. _____
 Address 1140 East Riverside Apt 10 Estimated Cost \$ 2500
 Type of work: New Service change _____ Rewiring _____ Additional Wiring _____ Temp. Elec. Req. _____
 Size of service 200 Undergound Overhead _____ No. of new circuits 30
 Description of work: New construction

PLUMBING: Plumbing Contractor Yoder Plumb & Htg Pn. 335-4240
 Address 1205 N. OTTOLEE Wausau WI Estimated Cost \$ 4800⁰⁰
 Water Tap Req. Size 3/4 Type of Pipe 3/4 K copper Water Dist. Pipe 1/2 copper
 San. Sewer Tap Req. Size 6" Type of Pipe PVCSDR 35 Dr. Waste Vt. Pipe PVC DWV-40
 St. Sewer Tap Req. _____ Size _____ Type of Pipe _____ Street to be Opened _____
 Main Building Drain Size 4" Main Vent Pipe Size 3" List Number of Plumbing Fixtures Below
 Water Closets 3 Bathtubs 1 Showers 1 Lavatories 3 Kitchen Sinks 1 Disposal 1 Dishwasher 1 Clothes Washer 1
 Floor Drains 0 Other Fixtures: Type domestic water heater No. 1
 Description of Work: New construction

MECHANICAL: Mechanical Contractor Yoder Plumb & Htg Pn. 335-4240
 Address 1205 N. OTTOLEE Wausau WI Estimated Cost 5300⁰⁰
 Heating System: Forced Air Gravity _____ Hot Water _____ Steam _____ Unit Heaters _____ Radiant _____ Baseboard _____
 Type of Fuel: Electric _____ Natural Gas Propane _____ Wood _____ Coal _____ Solar _____ Geothermal _____ Other _____
 No. of Heat Zones 1 Hot Water: (One Pipe _____ Two Pipe _____ Series Loop _____) Electric Heat: (No of Circuits _____) No. of Furnaces _____
 No. of Hot Air Runs 14 No. of Hot Water Radiators _____ Total Heat Loss 55,271 Rated Capacity of Furnace/Boiler 80,000
 Location of Heating Units: Crawl Space Floor Level _____ Attic _____ Suspended _____ Roof _____ Outside _____ Other _____
 Description of Work New ^{H.V.A.C.} HIPC system

DRAWINGS REQUIRED: All Applications must be Accompanied by Two Complete sets of Drawings Including SITE PLAN, FOUNDATION PLAN, FLOOR PLANS, STRUCTURAL FRAMING PLANS, EXTERIOR ELEVATIONS, SECTIONS and DETAILS, STAIR DETAILS, ELECTRICAL LAYOUT, PLUMBING ISOMETRIC, HEATING LAYOUT ETC. All plans shall be DRAWN TO SCALE. Show all existing structures on the site plan also, show Electric Panel and Furnace Locations.

READ AND SIGN BELOW; The undersigned hereby makes application for a permit for all work described herein, and agrees to complete the work in strict accordance with all applicable provisions of the current edition of the C.A.B.D. Building Code, the Napoleon Building and Zoning Codes, the Napoleon Engineering Dept. Rules and Regulations, Standard Specifications and other Pertinent Sections of the Napoleon Code of Ordinances.
 Date 1-4-90 Signature of Applicant Joseph A. Celani
 Application not valid without signature

RESIDENTIAL PLAN CORRECTION SHEET

CITY OF NAPOLEON
255 West Riverview Ave.
Napoleon, Ohio 43545
419/592-4010

ADDENDUM TO Permit No. 01901
Owner JOE CELANI
Contractor PAUL CELANI
Location 960 HARMONY DR.

Please note the items checked below and incorporate them into your plans as indicated: PERMIT NOT YET ISSUED, CORRECT PLANS AND RE-SUBMIT. PERMIT ISSUED, INCORPORATE ITEMS DURING CONSTRUCTION.

GENERAL			
<input checked="" type="checkbox"/>	Provide approved smoke detector(s) as req'd.		Show size of members supporting porch roof.
	Provide 1/2" gypsum wallboard between dwelling and garage, on garage side.		Provide double top plate for all bearing partitions and exterior walls.
<input checked="" type="checkbox"/>	Provide min. 1 3/8" solid wood door from garage to dwelling. (or equal)		Provide design data for prefab wood truss.
	Submit fully dimensioned plot plan.		Ceiling joists undersized in _____.
	Provide min. of 1-3'0" x 6'8" exit door.		Roof rafters undersized in _____.
<input checked="" type="checkbox"/>	Provide min. 22" x 30" attic access opening.		PLUMBING AND MECHANICAL
<input checked="" type="checkbox"/>	Provide min. 18" x 24" crawl space access opening.	<input checked="" type="checkbox"/>	Terminate all exhaust systems to outside air.
	Provide approved sheathing or flashing behind masonry veneer.	<input checked="" type="checkbox"/>	Insulate ducts in unheated areas.
	Provide min. 15# underlayment on roof.		Provide backflow prevention device on all hose bibs.
	Provide adequate fireplace hearth.		Terminate pressure and temperature relief valve drain in an approved manner.
	Install factory built fireplaces/stoves according to manufacturers instructions.		Provide dishwasher drain with approved air gap device.
	Terminate chimney 2' above roof or 2' above highest point of building within 10' of chimney.		METAL VENEERS
	LIGHT AND VENTILATION		Contact City Utilities Dept. to remove conductors and/or meter.
<input checked="" type="checkbox"/>	Provide mechanical exhaust or window in bathroom		Provide approved system of grounding and bonding.
	Provide min. <u>1170</u> Sq. In. net free area attic ventilation. <u>112 @ RIDGE</u>		ELECTRICAL
<input checked="" type="checkbox"/>	Provide min. <u>177</u> Sq. In. net free area crawl space ventilation. <u>W/ VAPOR BARR + FOUNDATION VENT 3' FR. CORNERS</u>	<input checked="" type="checkbox"/>	Show location of service entrance panel and service equipment panel.
	Min. depth of foundation below finished grade is 32".	<input checked="" type="checkbox"/>	G. F. C. I. req'd. on temporary electric.
	Min. size of footer _____" x _____".	<input checked="" type="checkbox"/>	Outdoor, bathroom and garage receptacles shall be protected by G. F. C. I.
<input checked="" type="checkbox"/>	Provide anchor bolts 1/2" @ 6' o.c. 1' from each corner. Embedded 7" in concrete and 15" in masonry.	<input checked="" type="checkbox"/>	Maximum number of receptacles permitted on a G. F. C. I. circuit shall be 10 for 20 A. circuits and 7 for 15A. circuits.
	Show size of basement columns.	<input checked="" type="checkbox"/>	Refrigerators, microwaves, washers, disposal, furnace and air conditioners shall be on separate circuits.
	FRAMING		INSPECTIONS
	Show size of wood girder in _____.		The following indicated inspections are required. The owner or his agent shall contact the City Building Dept. at least 24 hrs prior to the time the inspection is to be made.
	Provide design data for structural member in _____.	<input checked="" type="checkbox"/>	Footers and Setbacks.
	Floor joists undersized in _____.	<input checked="" type="checkbox"/>	Building sewer.
	Provide double joists under parallel bearing partitions.	<input checked="" type="checkbox"/>	Foundation.
<input checked="" type="checkbox"/>	Provide 1" x 4" let in corner bracing, approved sheathing, or equal.	<input checked="" type="checkbox"/>	Plumbing rough-in.
	Show size of headers for openings over 4' wide _____.	<input checked="" type="checkbox"/>	HVAC rough-in.
		<input checked="" type="checkbox"/>	Plumbing final.
		<input checked="" type="checkbox"/>	Final Building other.
		<input checked="" type="checkbox"/>	Electrical service.
		<input checked="" type="checkbox"/>	Electrical rough-in.
		<input checked="" type="checkbox"/>	Electrical final
			<u>BUILDING FRAMING</u>

Additional Corrections. _____

The approval of plans and specifications does not permit the violation of any section of the Building Code or other City Ordinance. This addendum is attached to Permit No. 01901 and made a part thereof. DATE APPROVED OR DISAPPROVED 1-9-90 Checked by ELDON HUBER
Plan Examiner.

DATE CHECKED AND APPROVED _____

Checked by _____

SEWER TAPPING PERMIT

Issued by

The City of Napoleon Engineering Dept.

255 West Riverview Ave. Napoleon, Ohio 43545 Pn. 592-4010

Entry No. _____

Permit No. SA 0127 Issued 1-5-90 Build. Permit No. 01901

Permit Fee \$ 60.00

Job Location 960 Harmony Drive

Street Bond \$ NONE

Lot 34 Back yard

Date Paid _____

Issued By E sub div. or legal disc.

Owner Joe Celani Pn. 599-3464

Address 1140 Riverview Apt 10

Agent PAUL COLAKI Pn. 337-3902

Address 439 W. ELM WADSWON OHIO

for office use only

WORK INFORMATION

Sanitary Sewer Tap 0 Size of Tap 6 Size and Type of Sewer A.S.T.M 3039 SDR 35 Street to be Opened yes no

Storm Sewer Tap X Size of Tap 4 Size and Type of Sewer P.U.C. Street to be Opened yes no

Street opening Agreement Approval Date _____ Opening Bond Fee (Set by Engineer) _____

READ AND SIGN BELOW; The undersigned hereby agrees complete the work described above and to make use of said sewers only as allowed by and in strict accordance with all applicable provisions of The Napoleon Engineering Dept. Rules and Regulations, The Napoleon Standard Specification for Water Main, Sanitary Sewer and Storm Sewer Construction and other Pertinent Sections of the Napoleon Code of Ordinances.

Date 1-4-90 Signature of Applicant Joseph U. Celani Permit not valid without signature

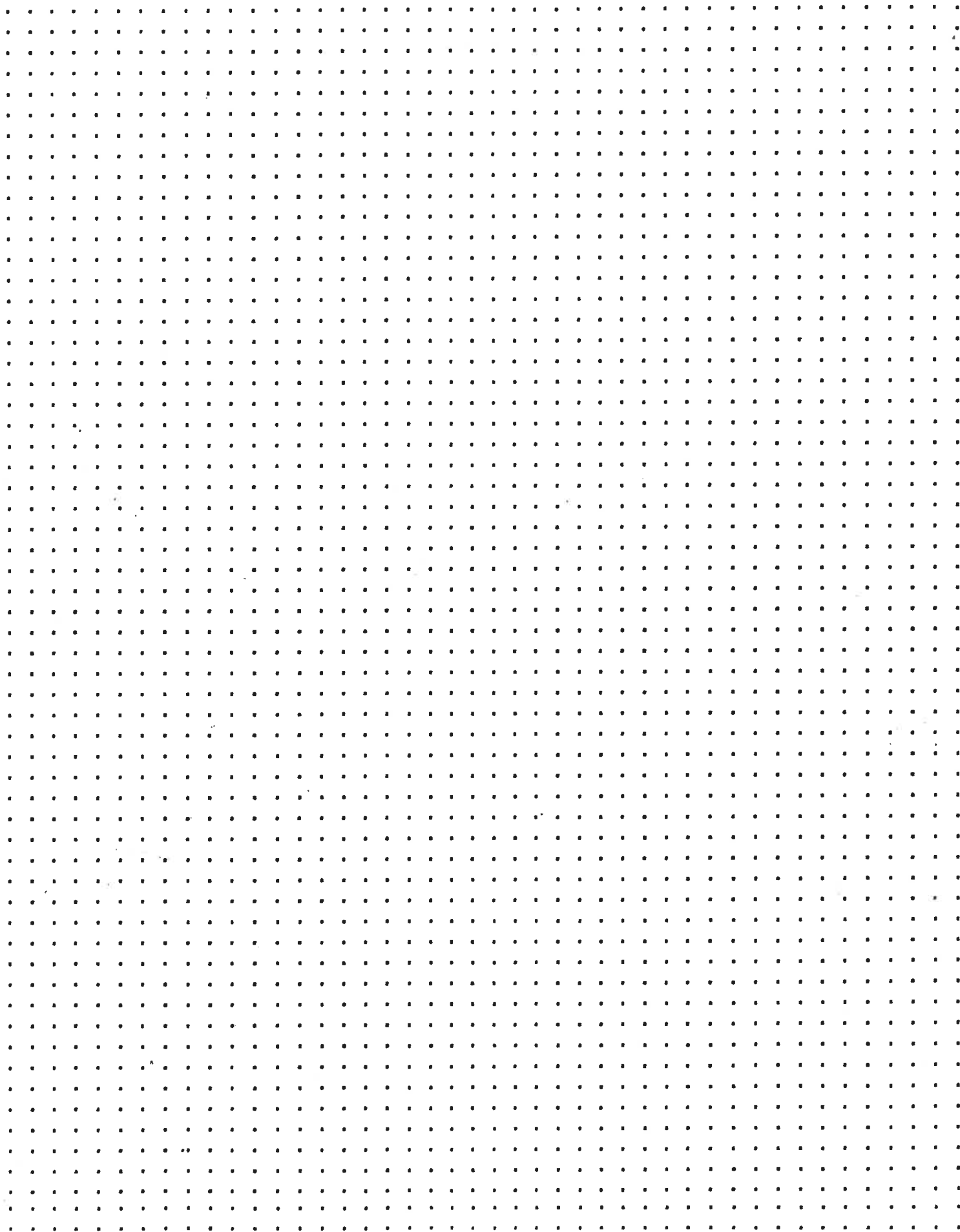
INSPECTION RECORD _____ to be completed by the Field Inspector

Date Inspection is made 4-2-90 Size and Type of Sewer 6" S.D.R. 35 A.S.T.M 3039

Location SEE PLAN Depth SEE PLAN Type of Test None Additional Information _____

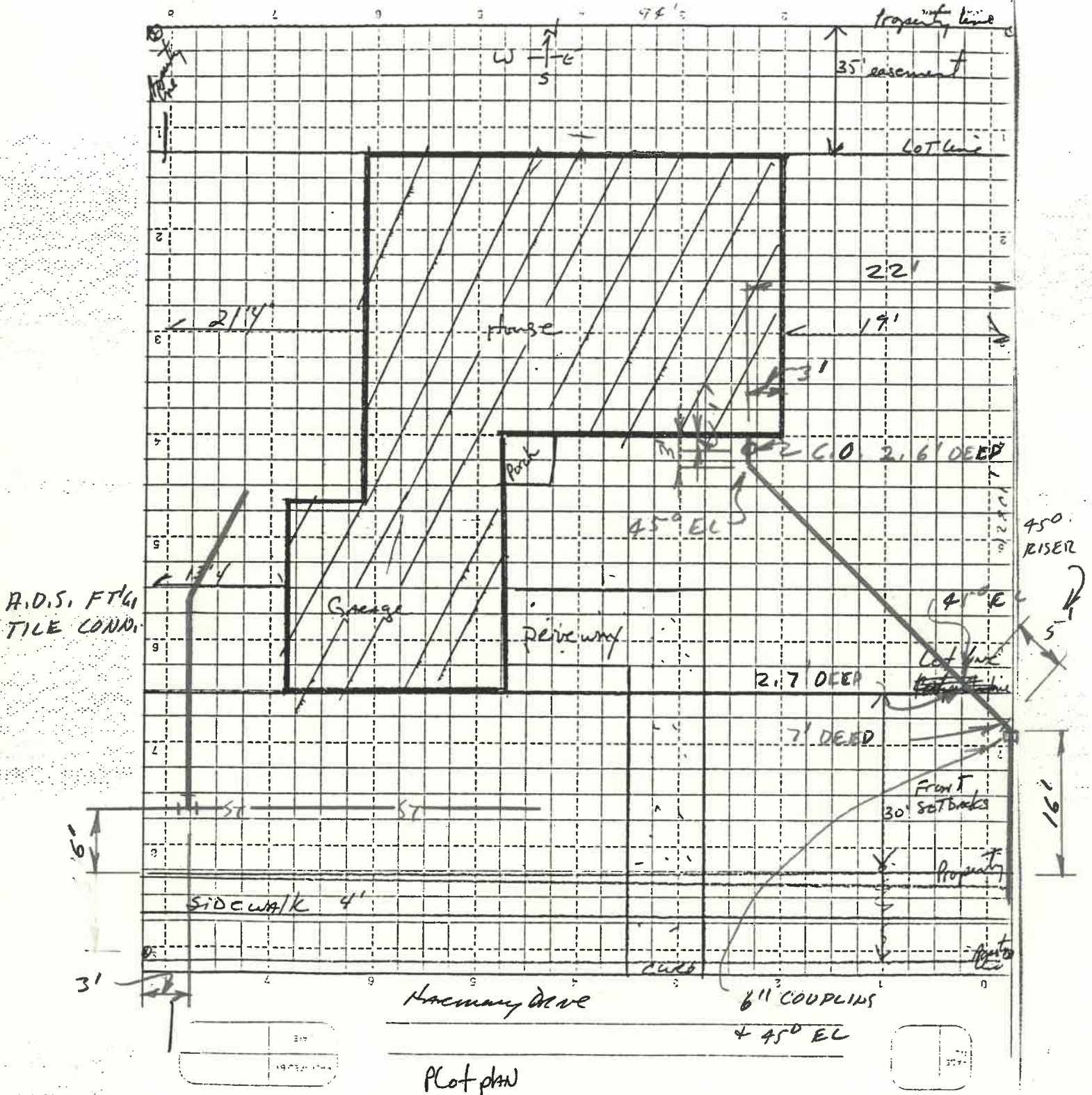
Date 4-2-90 Inspected By ELOON HUBER signature of inspector

Sketch Of Installation on Back or Attached



6" P.V.C. A.S.T.M. 3034
 BEDDED IN #57 STONE

GLUE JOINT AT 4" TO 6" REDUCER @ WALL LINE



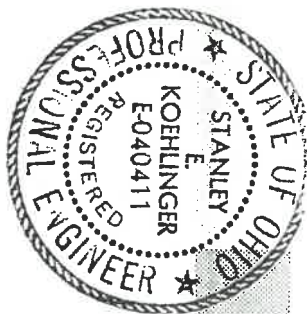
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STRESS INDICES MSI = 0.798 VSI = 0.533

2 BEAMS SIZE: 1.75 X 14.250 G13100-2.0

SLENDERNESS RATIO = 4.07 LIMIT = 10.0

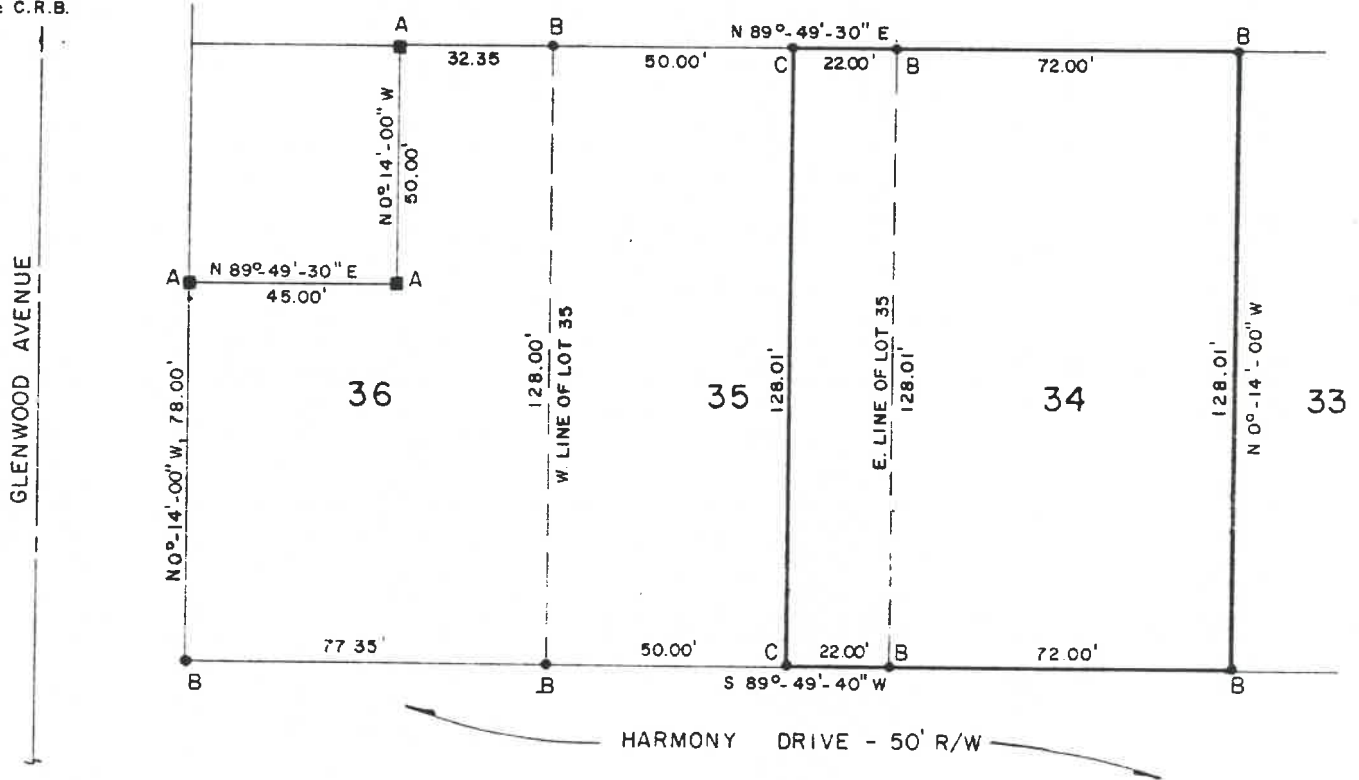
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REPLAT OF LOTS 34, 35, 36, BRICKYARD SUBDIVISION

PLAT OF SURVEY

SURVEY OF LOT 34 AND THE EAST 22 FEET OF LOT 35
OF BRICKYARD SUBDIVISION - PHASE I IN THE CITY OF
NAPOLEON, HENRY COUNTY, OHIO.

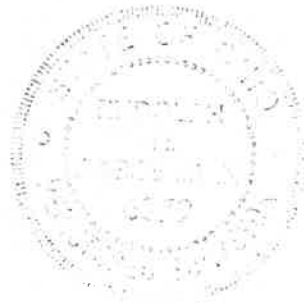


LEGEND:

- A - FOUND CONC MONUMENT
- B - FOUND IRON PIN.
- C - PLACED IRON PIN.

RECORD:

SURVEY FILED IN VOLUME 20, PAGE 157 OF THE
RECORD OF SURVEYS IN THE OFFICE OF THE HENRY
COUNTY ENGINEER, NAPOLEON, OHIO.



RUDOLPH H. BASSELMAN
Engineering & Surveying
1108 S. Defiance St Archbold, OH 43502
(419) 446-2643

Rudolph H. Basselman

Rudolph H. Basselman P.S. 6379
Professional Surveyor

MEMBER	FORCE (LBS)	CHORD	SLOPE/12	DEPTH IN	LOAD (PLF)	MAXIMUM MEMBER LENGTH	MEMBER FORCE (LBS)	CONC LOAD	CHORDS	SIZE	LUMBER DESCRIPTION	DESIGN CRITERIA
1 HL01 GNO20 3.0X 5.0	21170	8-1-14	5.000	70.0	0.0	2-8	516C	1	1-3	2X 4	NO.1 KD15 SO. PINE	TOP CH. L1= 25 PSF
2 HL11 GNO20 1.0X 4.0	1786C	7-6-2	5.000	70.0	0.0	3-8	781T	3-5	2X 4	NO.1 KD15 SO. PINE	DI= 10 PSF	
3 HL02 GNO20 4.0X 4.0	1315C	7-6-2	-5.000	70.0	0.0	3-7	67T	5-1	2X 4	NO.1 KD15 SO. PINE	BOT CH. L1= 10 PSF	
4 HL01 GNO20 3.0X 5.0	1185T	8-1-14	5.000	70.0	0.0	4-7	145T	2-8	2X 4	NO.3 S.P.F.	DI= 10 PSF	
5 HL01 GNO20 2.5X 4.0	1094C	3-6-0	0.000	20.0	5.3	4-6	2750C	2-8	2X 4	NO.2 KD15 SO. PINE	DI= 10 PSF	
6 IN11 GNO20 3.0X 4.0	1128T	7-1-15	0.000	20.0	10.0	4-6		4-6	2X 4	NO.2 KD15 SO. PINE	DI= 10 PSF	
7 IN02 GNO20 3.0X 4.0	1173T	10-0-2	0.000	20.0	10.0						DI= 10 PSF	
8 IN02 GNO20 3.0X 4.0	1954T	10-7-15	0.000	20.0	6.3						DI= 10 PSF	

DESIGN SPEC. FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES, TPI, 1985

PROVIDE ANCHORAGE AT BEARING JT- 1 FOR 176 LBS UPLIFT PROVIDE ANCHORAGE AT BEARING JT- 6 FOR 432 LBS UPLIFT PROVIDE FOR 30.0 LBS HORIZONTAL REACTION AT JOINT- 1

THIS TRUSS HAS BEEN DESIGNED FOR THE WIND LOADS GENERATED BY 90.0 M.P.H. WINDS AT 25.0 FT. ABOVE GROUND LEVEL, USING 7.0 P.S.F. TOP CHORD DEAD LOAD AND 10.0 P.S.F. BOTTOM CHORD DEAD LOAD, 100.0 MILES FROM HERRICKS OCEANLINE, ON A CATEGORY I ENCLOSED BUILDING, OF DIMENSIONS 99.0 BY 28.0 WITH EXPOSURE C (ANSI A58.1-1982).

MAX. PURLIN SPACE- 0.0 FT. MAX. UNBRACED BOT. CH. LEN.- 5.3 FT. *** PLYWOOD SHEATHING REQUIRED ON TOP CHORD *** 1-1X4 LAT. BRACE REQD. AT 1/2 LEN. WEBS 3-8 4-6

NOTE: LATERAL BRACES AND PURLINS INDICATED FOR TRUSS MEMBERS ARE REQUIRED TO REDUCE BUCKLING LENGTH OF MEMBER, AND SHOULD BE NAILLED TO TRUSS MEMBERS WITH MINIMUM OF 2-LD COMMON WIRE NAILS. PROVISIONS MUST BE MADE AT ENDS OR SPECIFIED INTERVALS TO RESTRAIN OR ANCHOR LATERAL BRACING. BY OTHERS.

THIS TRUSS IS DESIGNED TO SUPPORT VERTICAL LOADS AS DETERMINED BY OTHERS AND SHOWN ON INPUT LISTING, VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND BRACING OR OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED, IS THE RESPONSIBILITY OF THE PROJECT ARCHITECT OR ENGINEER.

MAIL VALDES (PST) GROSS CHORDS WEBS

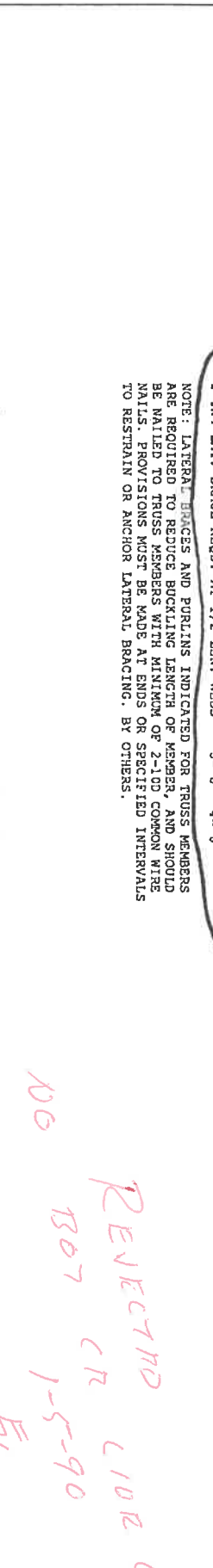
MAX MIN MAX MIN

GNO20 228 180 228 140

RIGHT GANTTLEVER- 3-4-0

LEFT OVERHANG- 2-0-0

RIGHT OVERHANG- 2-0-0



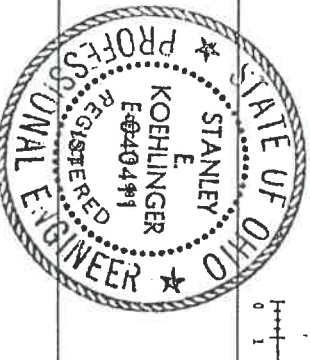
89-5113-1

MEMBER	FORCE (LBS)	CHORD	SLOPE/12	DEPTH IN	LOAD (PLF)	MAXIMUM MEMBER LENGTH	MEMBER FORCE (LBS)	CONC LOAD	CHORDS	SIZE	LUMBER DESCRIPTION	DESIGN CRITERIA
1 HL01 GNO20 3.0X 5.0	21170	8-1-14	5.000	70.0	0.0	2-8	516C	1	1-3	2X 4	NO.1 KD15 SO. PINE	TOP CH. L1= 25 PSF
2 HL11 GNO20 1.0X 4.0	1786C	7-6-2	5.000	70.0	0.0	3-8	781T	3-5	2X 4	NO.1 KD15 SO. PINE	DI= 10 PSF	
3 HL02 GNO20 4.0X 4.0	1315C	7-6-2	-5.000	70.0	0.0	3-7	67T	5-1	2X 4	NO.1 KD15 SO. PINE	BOT CH. L1= 10 PSF	
4 HL01 GNO20 3.0X 5.0	1185T	8-1-14	5.000	70.0	0.0	4-7	145T	2-8	2X 4	NO.3 S.P.F.	DI= 10 PSF	
5 HL01 GNO20 2.5X 4.0	1094C	3-6-0	0.000	20.0	5.3	4-6	2750C	2-8	2X 4	NO.2 KD15 SO. PINE	DI= 10 PSF	
6 IN11 GNO20 3.0X 4.0	1128T	7-1-15	0.000	20.0	10.0	4-6		4-6	2X 4	NO.2 KD15 SO. PINE	DI= 10 PSF	
7 IN02 GNO20 3.0X 4.0	1173T	10-0-2	0.000	20.0	10.0						DI= 10 PSF	
8 IN02 GNO20 3.0X 4.0	1954T	10-7-15	0.000	20.0	6.3						DI= 10 PSF	

THIS TRUSS IS DESIGNED TO SUPPORT VERTICAL LOADS AS DETERMINED BY OTHERS AND SHOWN ON INPUT LISTING, VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND BRACING OR OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED, IS THE RESPONSIBILITY OF THE PROJECT ARCHITECT OR ENGINEER.

MEMBER	FORCE (LBS)	CHORD	SLOPE/12	DEPTH IN	LOAD (PLF)	MAXIMUM MEMBER LENGTH	MEMBER FORCE (LBS)	CONC LOAD	CHORDS	SIZE	LUMBER DESCRIPTION	DESIGN CRITERIA
1 HL01 GNO20 3.0X 5.0	21170	8-1-14	5.000	70.0	0.0	2-8	516C	1	1-3	2X 4	NO.1 KD15 SO. PINE	TOP CH. L1= 25 PSF
2 HL11 GNO20 1.0X 4.0	1786C	7-6-2	5.000	70.0	0.0	3-8	781T	3-5	2X 4	NO.1 KD15 SO. PINE	DI= 10 PSF	
3 HL02 GNO20 4.0X 4.0	1315C	7-6-2	-5.000	70.0	0.0	3-7	67T	5-1	2X 4	NO.1 KD15 SO. PINE	BOT CH. L1= 10 PSF	
4 HL01 GNO20 3.0X 5.0	1185T	8-1-14	5.000	70.0	0.0	4-7	145T	2-8	2X 4	NO.3 S.P.F.	DI= 10 PSF	
5 HL01 GNO20 2.5X 4.0	1094C	3-6-0	0.000	20.0	5.3	4-6	2750C	2-8	2X 4	NO.2 KD15 SO. PINE	DI= 10 PSF	
6 IN11 GNO20 3.0X 4.0	1128T	7-1-15	0.000	20.0	10.0	4-6		4-6	2X 4	NO.2 KD15 SO. PINE	DI= 10 PSF	
7 IN02 GNO20 3.0X 4.0	1173T	10-0-2	0.000	20.0	10.0						DI= 10 PSF	
8 IN02 GNO20 3.0X 4.0	1954T	10-7-15	0.000	20.0	6.3						DI= 10 PSF	

IMPORTANT: READ ALL NOTES ON THIS DRAWING!



11
DEC 19, 1989 F3-19F

WARNING- VERIFY YOUR INPUT TO AVOID DESIGN AND FABRICATION MISTAKES.
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REQUEST NO. CALCONLY QUOTE MAUSEON WOODWORKING (545624) TG

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13 7
REQUEST NO. CALCONLY QUOTE MAUSEON WOODWORKING (545624) TG T-1
DEC 19, 1989 F3-19F

SPAN (OUT TO OUT) 28.000
NO. OF JOINTS 8
LOC. OF REACTIONS 1 6
SHORT TERM INCREASE TCH 1.15 BCH 1.15 WEBS 1.15 PLATES 1.15
TOP CHORD LOAD SHARE = 15 BOTTOM CHORD LOAD SHARE = 15

PLYWOOD SHEATHING USED ON TOP CHORD WHERE APPLICABLE.

TRUSS GEOMETRY AND LOAD DATA	VERTICAL	VERTICAL
UT HOR.DISP. FT	VER.DISP. FT	UNIF.LD. CONC.LD.
1 8.1583	5.0000	PLF LBS
2 7.5083	-70.00	-140.00
3 7.5083	-5.0000	0.00
4 8.1583	-70.00	0.00
5 -3.5000	0.0000	-20.00
6 -7.1611	0.0000	-20.00
7 -10.0111	0.0000	0.00
8 -10.6611	0.0000	-20.00

TOTAL POSITIVE DISPLACEMENT= 31.33

NO. OF WEBS= 5
2- 8 3- 8 3- 7 4- 7 4- 6

GROSS REACTIONS (LBS):
RV-1= 1355.1 RV-6= 1744.9
RH-1= 0.0

KEM	FORCE LBS	MID IN.	DEP IN.	FB PSI	FC:FT PSI	Q	P/AF	VM/ZF	HM/ZF	CSI	LAT. BRC
TOP CHORD MEMBERS											
1- 2	-2117.	1.50	3.50	2415	1668	0.90	0.24	0.65	0.00	0.89	0.0
2- 3	-1786.	1.50	3.50	2415	1668	0.90	0.20	0.65	0.00	0.85	0.0
3- 4	-1315.	1.50	3.50	2415	1668	0.90	0.15	0.65	0.00	0.80	0.0
4- 5	1185.	1.50	3.50	2415	1208	0.90	0.19	0.65	0.00	0.84	0.0
BOT CHORD MEMBERS											
5- 6	-1094.	1.50	3.50	1825	968	1.00	0.22	0.22	0.00	0.44	5.3
6- 7	1128.	1.50	3.50	2013	1035	1.00	0.21	0.40	0.00	0.60	10.0
7- 8	1173.	1.50	3.50	2013	1035	1.00	0.22	0.50	0.00	0.72	10.0
8- 1	1994.	1.50	3.50	2013	1035	1.00	0.36	0.50	0.00	0.86	10.0

DEFLECTION AT 8 = -0.1709 INCHES
DEFLECTION BETWEEN 1- 8 = -0.4651 INCHES

EXPLANATIONS:
P/AF = AXIAL FORCE DIVIDED BY THE CROSS-SECTIONAL AREA AND THE ALLOWABLE UNIT STRESS IN TENSION OR COMPRESSION.
VM/ZF= VERTICAL BENDING MOMENT DIVIDED BY THE SECTION MODULUS AND THE ALLOWABLE UNIT STRESS IN BENDING.
HM/ZF= HORIZONTAL BENDING MOMENT DIVIDED BY THE SECTION MODULUS AND THE ALLOWABLE UNIT STRESS IN BENDING.
CSI = COMBINED STRESS INTERACTION EQUATION IS THE P/AF + VM/ZF + HM/ZF
LAT. = MAXIMUM DISTANCE ALLOWABLE (FT) WITHOUT REQUIREMENT OF SUPPORT.
STRESSES SHOWN (FB FT FC) ARE ALLOWABLE LUMBER STRESSES BY THE SHORT TERM INCREASE AND OTHER APPROPRIATE FACTORS WHEREVER APPLICABLE.

CHORDS	SIZE	LUMBER DESCRIPTION
1- 3	2X 4	NO.1 KD15 SO. PINE
3- 5	2X 4	NO.1 KD15 SO. PINE
5- 1	2X 4	NO.2 KD15 SO. PINE
WEBS	2X 4	NO.3 S.P.F.
2- 8	3- 8	3- 7
WEBS	2X 4	NO.2 KD15 SO. PINE
4- 6		



MEMBR	FORCE (LBS)	HOR DISP	SLOPE/12	DEPTH IN	LOAD (PLF)	LENGTH	MEMBR FR-TO	W E B S FORCE (LBS)	CONC JT	LOAD LBS	CHORDS	SIZE	LUMBER DESCRIPTION	DESIGN CRITERIA
1	2590C	8-1-14	5.000	70.0	0.0	2-7	510C	1	140	3-5	2X 4	NO.1 KD15 SO. PINE	TOP CH. DL= 25 PSF	
2	2263C	7-6-2	5.000	70.0	0.0	3-7	772T	1	140	3-5	2X 4	NO.1 KD15 SO. PINE	DL= 10 PSF	
3	2263C	7-6-2	-5.000	70.0	0.0	3-6	772T	1	140	3-5	2X 4	NO.1 KD15 SO. PINE	DL= 10 PSF	
4	2590C	8-1-14	-5.000	70.0	0.0	4-6	510C	1	140	3-5	2X 4	NO.1 KD15 SO. PINE	DL= 10 PSF	
5	2391T	10-7-15	0.000	20.0	10.0	4-6	510C	1	140	3-5	2X 4	NO.1 KD15 SO. PINE	DL= 10 PSF	
6	2391T	10-7-15	0.000	20.0	10.0	4-6	510C	1	140	3-5	2X 4	NO.1 KD15 SO. PINE	DL= 10 PSF	
7	1618T	10-7-15	0.000	20.0	10.0	4-6	510C	1	140	3-5	2X 4	NO.1 KD15 SO. PINE	DL= 10 PSF	
8	2391T	10-7-15	0.000	20.0	10.0	4-6	510C	1	140	3-5	2X 4	NO.1 KD15 SO. PINE	DL= 10 PSF	
9	2391T	10-7-15	0.000	20.0	10.0	4-6	510C	1	140	3-5	2X 4	NO.1 KD15 SO. PINE	DL= 10 PSF	

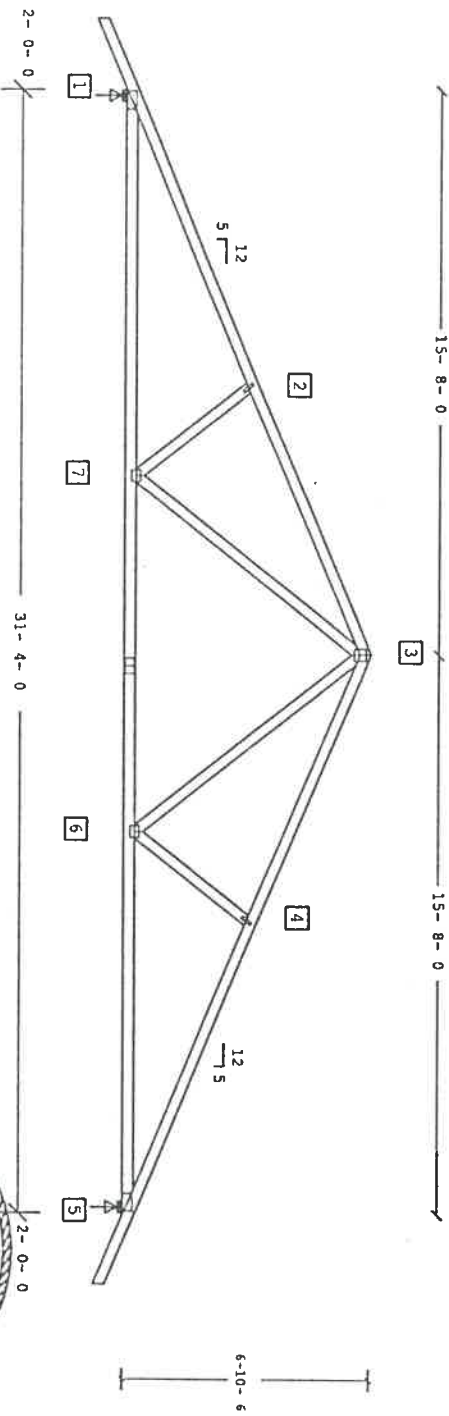
NOTE: LATERAL BRACES AND PURLINS INDICATED FOR TRUSS MEMBERS ARE REQUIRED TO REDUCE BUCKLING LENGTH OF MEMBER, AND SHOULD BE MAILED TO TRUSS MEMBERS WITH MINIMUM OF 2-100 COMMON WIRE NAILS. PROVISIONS MUST BE MADE AT ENDS OR SPECIFIED INTERVALS TO RESTRAIN OR ANCHOR LATERAL BRACING. BY OTHERS.

MAX. PURLIN SPACING= 10.0 FT., MAX. UNBRACED BOT. CH. LEN.= 10.0 FT.

*** PLYWOOD SHEATHING REQUIRED ON TOP CHORD ***

THIS TRUSS IS DESIGNED TO SUPPORT VERTICAL LOADS AS DETERMINED BY OTHERS AND SHOWN ON INPUT LISTING. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND BRACING OR OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED, IS THE RESPONSIBILITY OF THE PROJECT ARCHITECT OR ENGINEER.

LEFT OVERHANG = 2-0-0
RIGHT OVERHANG = 2-0-0

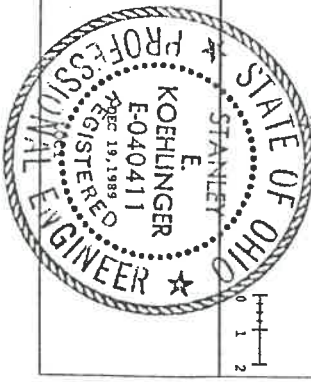


GROSS BRG
JT REACT 1N-SX 4-0
1 1550 4-0
5 1550 4-0

CAMBER= 0-2/8 HEEL = 4/16

Handling & Erection	Miscellaneous Information	Bracing Information	Connector Hardware	Lumber
Caution handling of components shall not be permitted. Plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after bracing and lashing are complete. The use of this component shall be specified by the designer of the component. Lateral bracing shall be applied to the component. Care must be exercised to install components at proper bearing points, not on top, and properly braced. Read all instructions for the component. Gang-Nail Systems, Inc. is not responsible for the fabrication, handling, shipment and installation of components.	The data sheet and the information hereon is the property of Gang Nail Systems, Inc. and its application in any other project or in any other way is prohibited. The use of this component shall be specified by the designer of the component. Lateral bracing shall be applied to the component. Care must be exercised to install components at proper bearing points, not on top, and properly braced. Read all instructions for the component. Gang-Nail Systems, Inc. is not responsible for the fabrication, handling, shipment and installation of components.	All lateral bracing specified is for bracing structural web members and is to be applied to both faces of the member with both lath and end plates. Bracing members are assumed to be equally spaced along web length. Lateral bracing shall be applied to the member at the top and bottom. Restraint of lateral bracing and end plates shall be provided by the designer of the complete structure.	Connector plates are manufactured in accordance with TPI. Plates must be of the same thickness and material as the member being connected. Plates must be of the size, gauge and capacity shown. The AnchorTite lath shall be used for all connections. Position plates shall be as shown.	Lumber must bear a grade mark from an approved inspection bureau and must be shown and equal to or better than the grade specified. Design Criteria: The design and the materials specified are in accordance with the latest revision of NDS, AITC and TPI.

IMPORTANT: READ ALL NOTES ON THIS DRAWING!



11
DEC 19, 1989 F3-19F

WARNING- VERIFY YOUR INPUT TO AVOID DESIGN AND FABRICATION MISTAKES.
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REQUEST NO. CALCONLY QUOTE WAUSEON WOODWORKING (545624) TG
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13 7
REQUEST NO. CALCONLY QUOTE WAUSEON WOODWORKING (545624) TG T-2
DEC 19, 1989 F3-19F

SPAN (OUT TO OUT) 31.333
NO. OF JOINTS 7
LOC. OF REACTIONS 1 5
SHORT TERM INCREASE TCH 1.15 BCH 1.15 WEBS 1.15 PLATES 1.15

TOP CHORD LOAD SHARE = 15 BOTTOM CHORD LOAD SHARE = 15
PLYWOOD SHEATHING USED ON TOP CHORD WHERE APPLICABLE.

TRUSS GEOMETRY AND LOAD DATA	VER. DISP.	VERTICAL UNIT.LD.	VERTICAL CONC.LD.
JT HOR.DISP. FT	SLOPE/12	PLF	IBS
1	8.1583	5.0000	-70.00
2	7.5083	5.0000	-70.00
3	7.5083	-5.0000	-70.00
4	8.1583	-5.0000	-70.00
5	-10.6611	0.0000	-20.00
6	-10.0111	0.0000	-20.00
7	-10.6611	0.0000	-20.00

TOTAL POSITIVE DISPLACEMENT= 31.33

NO. OF WEBS= 4
2- 7 3- 7 3- 6 4- 6

GROSS REACTIONS(LBS):
RV- 1= 1550.0 RV- 5= 1550.0
RH- 1= 0.0

MEM	FORCE LBS	WTD IN.	DEP IN.	FB PSI	FC:FT PSI	Q	P/AF	VM/ZF	HM/ZF	CSI	LAT. BRC
1- 2	-2590.	1.50	3.50	2415	1668	0.90	0.30	0.65	0.00	0.94	0.0
2- 3	-2263.	1.50	3.50	2415	1668	0.90	0.26	0.65	0.00	0.91	0.0
3- 4	-2263.	1.50	3.50	2415	1668	0.90	0.26	0.65	0.00	0.91	0.0
4- 5	-2590.	1.50	3.50	2415	1668	0.90	0.30	0.65	0.00	0.94	0.0
5- 6	2391.	1.50	3.50	2013	1035	1.00	0.44	0.50	0.00	0.94	10.0
6- 7	1618.	1.50	3.50	2013	1035	1.00	0.30	0.50	0.00	0.80	10.0
7- 1	2391.	1.50	3.50	2013	1035	1.00	0.44	0.50	0.00	0.94	10.0

TOP CHORD MEMBERS

MEM	FORCE LBS	WTD IN.	DEP IN.	FB PSI	FC:FT PSI	Q	P/AF	VM/ZF	HM/ZF	CSI	LAT. BRC
1- 2	-2590.	1.50	3.50	2415	1668	0.90	0.30	0.65	0.00	0.94	0.0
2- 3	-2263.	1.50	3.50	2415	1668	0.90	0.26	0.65	0.00	0.91	0.0
3- 4	-2263.	1.50	3.50	2415	1668	0.90	0.26	0.65	0.00	0.91	0.0
4- 5	-2590.	1.50	3.50	2415	1668	0.90	0.30	0.65	0.00	0.94	0.0

BOT CHORD MEMBERS

MEM	FORCE LBS	WTD IN.	DEP IN.	FB PSI	FC:FT PSI	Q	P/AF	VM/ZF	HM/ZF	CSI	LAT. BRC
5- 6	2391.	1.50	3.50	2013	1035	1.00	0.44	0.50	0.00	0.94	10.0
6- 7	1618.	1.50	3.50	2013	1035	1.00	0.30	0.50	0.00	0.80	10.0
7- 1	2391.	1.50	3.50	2013	1035	1.00	0.44	0.50	0.00	0.94	10.0
2- 7	-510.	1.50	3.50	633	418	0.00	0.23	0.00	0.00	0.23	4.2
3- 7	-772.	1.50	3.50	633	374	0.00	0.39	0.00	0.00	0.39	8.2

CHORDS	SIZE	LUMBER DESCRIPTION
1- 3	2X 4	NO.1 KD15 SO. PINE
3- 5	2X 4	NO.1 KD15 SO. PINE
5- 1	2X 4	NO.2 KD15 SO. PINE

ALL WEBS 2X 4 NO.3 S.P.F.

DEFLECTION AT 6 = -0.2635 INCHES
DEFLECTION BETWEEN 6- 7 = -0.5687 INCHES

EXPLANATIONS:
P/AF = AXIAL FORCE DIVIDED BY THE CROSS-SECTIONAL AREA AND THE ALLOWABLE UNIT STRESS IN TENSION OR COMPRESSION.
VM/ZF = VERTICAL BENDING MOMENT DIVIDED BY THE SECTION MODULUS AND THE ALLOWABLE UNIT STRESS IN BENDING.
HM/ZF = HORIZONTAL BENDING MOMENT DIVIDED BY THE SECTION MODULUS AND THE ALLOWABLE UNIT STRESS IN BENDING.
CSI = COMBINED STRESS INTERACTION EQUATION IS THE ALLOWABLE P/AF + VM/ZF + HM/ZF
LAT. = MAXIMUM DISTANCE ALLOWABLE (FT) WITHOUT REDUCED STRESS SUPPORT.
BRC = STRESSES SHOWN (FB FT FC) ARE ALLOWABLE LUMBER STRESSES BY THE SHORT TERM INCREASE AND OTHER APPROPRIATE WHEREVER APPLICABLE.



MEMBER	FORCE (LBS)	CHORDS	SIZE	LONGER	DESCRIPTION	DESIGN CRITERIA
1-2	2590C	1-3	2X 4	NO.1 KD15 SO.	PINE	TOP CH. L ₁ = 25 PSF
2-3	2283C	3-5	2X 4	NO.1 KD15 SO.	PINE	DL= 10 PSF
3-4	2283C	5-1	2X 4	NO.2 KD15 SO.	PINE	BOT CH. L ₁ = 0 PSF
4-5	2590C					DL= 10 PSF
5-6	2391T					DL= 10 PSF
6-7	1618T					DL= 10 PSF
7-1	2391T					TOTAL LOAD= 45 PSF

DESIGN SPECS. FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES, TP1,1985

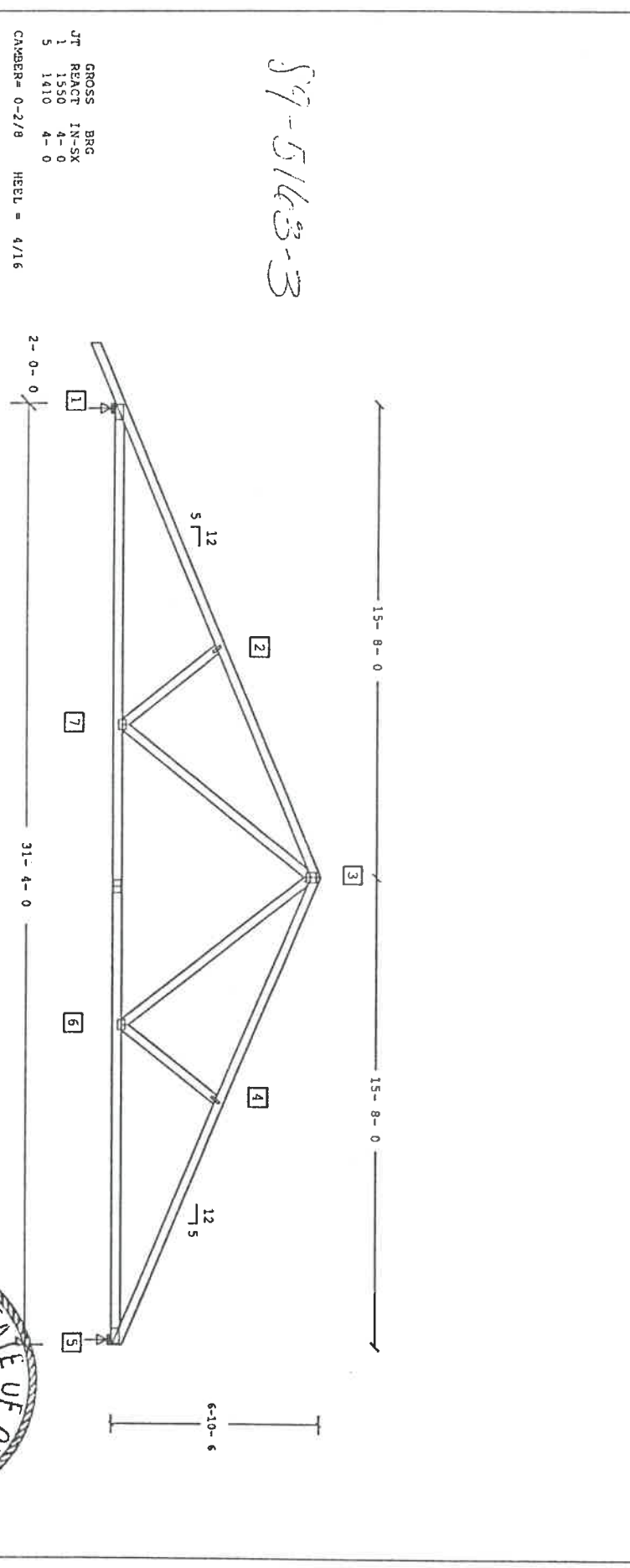
NOTE: LATERAL BRACES AND PURLINS INDICATED FOR TRUSS MEMBERS ARE REQUIRED TO REDUCE BUCKLING LENGTH OF MEMBERS, AND SHOULD BE NAILED TO TRUSS MEMBERS WITH MINIMUM OF 2-1/2" COMMON NAIL SPACING. PROVISIONS MUST BE MADE AT ENDS OR SPECIFIED INTERVALS TO RESTRAIN OR ANCHOR LATERAL BRACING, BY OTHERS.

MAX. PURLIN SPACING = 0.0 FT. MAX. UNBRACED BOT. CH. LEN. = 10.0 FT. *** PLYWOOD SHEATHING REQUIRED ON TOP CHORD ***

THIS TRUSS IS DESIGNED TO SUPPORT VERTICAL LOADS AS DETERMINED BY OTHERS AND SHOWN ON INPUT LISTING. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND BRACING OR OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED, IS THE RESPONSIBILITY OF THE PROJECT ARCHITECT OR ENGINEER.

INCREASES PER CENT: LDBR= 15 NAIL= 15 TCH L₁= 15 BR L₁= 15

NAIL VALUES (PSI) GROSS CHORDS WEBS MAX MIN GNO20 228 180 190 140 LEFT OVERHANG = 2-0-0



Handing & Erection: Careless handling of components shall not be permitted. Temporary and permanent bracing for loading components shall be provided. Care must be taken to insure that all components are installed by others. No loads are to be applied to the component until after all bracing and fastenings are complete. All no time shall be greater than design loads be applied to the truss. Care must be exercised to insure that all components are bearing parts, nift side up, and properly braced. Read all notes and design specifications carefully. No responsibility for the fabrication, handling, shipment and installation of components.

Miscellaneous Information: This data sheet and the information hereon is the property of Stark Truss Company, Inc. and is not to be copied in whole or in part or used for detailed or used for forming information. The use of this component shall be specified by the designer of the complete structure. Designer code compliance, approvals and instructions from the designer of the complete structure before construction. DO NOT USE THIS DESIGN. Stark Truss Company, Inc. is approving only the structural design of the unit shown on this drawing.

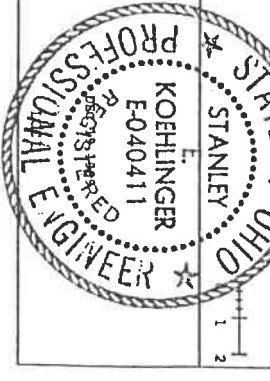
Bracing Information: All lateral bracing specified is for bracing individual web members and must be provided. Chords required are to be equally spaced along web length. Chord members are assumed to be laterally restrained by sheathing. Additional bracing for the overall structure is to be provided by the designer of the complete structure.

Connector Hardware: Connector plates are manufactured in accordance with TPI. Plates must be installed on both faces of the chord members. Plates must be of the size, grade and capacity shown. Refer to the Anchor Truss joint detail for details of connection of plates.

Lumber: Lumber must have a grade mark from an approved inspection bureau and must be of the size and species shown. Lumber shall be better than the grade specified.

Design Criteria: The design and the materials specified are in accordance with the provisions of NDS.

IMPORTANT: READ ALL NOTES ON THIS DRAWING!



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4

11
DEC 19, 1989 F3-19F

WARNING- VERIFY YOUR INPUT TO AVOID DESIGN AND FABRICATION MISTAKES.
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13 7
REQUEST NO. CALCONLY QUOTE WAUSEON WOODWORKING (545624) TG T-3
DEC 19, 1989 F3-19F

SPAN (OUT TO OUT) 31.333
NO. OF JOINTS 7
LOC. OF REACTIONS 1 5
SHORT TERM INCREASE TCH 1.15 BCH 1.15 WEBS 1.15 PLATES 1.15

TOP CHORD LOAD SHARE = 15 BOTTOM CHORD LOAD SHARE = 15
PLYWOOD SHEATHING USED ON TOP CHORD WHERE APPLICABLE.

TRUSS GEOMETRY AND LOAD DATA

JT	HOR. DISP. FT	VER. DISP. SLOPE/12	VERTICAL UNIF. LD. PLF	VERTICAL CONC. LD. LBS
1	8.1583	5.0000	-70.00	-140.00
2	7.5083	5.0000	-70.00	0.00
3	7.5083	-5.0000	-70.00	0.00
4	8.1583	-5.0000	-70.00	0.00
5	-10.6611	0.0000	-20.00	0.00
6	-10.6611	0.0000	-20.00	0.00
7	-10.6611	0.0000	-20.00	0.00

TOTAL POSITIVE DISPLACEMENT- 31.33

NO. OF WEBS= 4
2- 7 3- 7 3- 6 4- 6

GROSS REACTIONS(LBS):
RV- 1= 1550.0 RV- 5= 1410.0
RH- 1= 0.0

MEM	FORCE LBS	WID IN.	DEP IN.	FB PSI	FC:FT PSI	Q	P/AF	VM/ZF	HM/ZF	CSI	IAT. BRC
TOP CHORD MEMBERS											
1- 2	-2590.	1.50	3.50	2415	1668	0.90	0.30	0.65	0.00	0.94	0.0
2- 3	-2263.	1.50	3.50	2415	1668	0.90	0.26	0.65	0.00	0.91	0.0
3- 4	-2263.	1.50	3.50	2415	1668	0.90	0.26	0.65	0.00	0.91	0.0
4- 5	-2590.	1.50	3.50	2415	1668	0.90	0.30	0.65	0.00	0.94	0.0
BOT CHORD MEMBERS											
5- 6	2391.	1.50	3.50	2013	1035	1.00	0.44	0.50	0.00	0.94	10.0
6- 7	1618.	1.50	3.50	2013	1035	1.00	0.30	0.50	0.00	0.80	10.0
7- 1	2391.	1.50	3.50	2013	1035	1.00	0.44	0.50	0.00	0.94	10.0
WEB MEMBERS											
2- 7	-510.	1.50	3.50	633	418	0.00	0.23	0.00	0.00	0.23	4.2
3- 7	772.	1.50	3.50	633	374	0.00	0.39	0.00	0.00	0.39	8.2

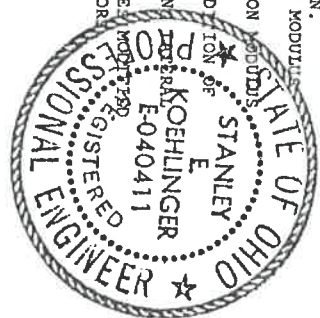
3- 6	772.	1.50	3.50	633	374	0.00	0.39	0.00	0.00	0.39	8.2
4- 6	-510.	1.50	3.50	633	418	0.00	0.23	0.00	0.00	0.23	4.2

DEFLECTION AT 6 = -0.2635 INCHES
DEFLECTION BETWEEN 6- 7 = -0.5587 INCHES

EXPLANATIONS:
P/AF - AXIAL FORCE DIVIDED BY THE CROSS-SECTIONAL AREA AND THE ALLOWABLE UNIT STRESS IN TENSION OR COMPRESSION.
VM/ZF - VERTICAL BENDING MOMENT DIVIDED BY THE SECTION MODULUS AND THE ALLOWABLE UNIT STRESS IN BENDING.
HM/ZF - HORIZONTAL BENDING MOMENT DIVIDED BY THE SECTION MODULUS AND THE ALLOWABLE UNIT STRESS IN BENDING.
CSI - COMBINED STRESS INTERACTION EQUATION IS THE ADDITION OF P/AF + VM/ZF + HM/ZF
LAT. - MAXIMUM DISTANCE ALLOWABLE (FT) WITHOUT REQUIRING BRC SUPPORT.
STRESSES SHOWN (FB FT FC) ARE ALLOWABLE LUMBER STRESSES BY THE SHORT TERM INCREASE AND OTHER APPROPRIATE FACTOR WHEREVER APPLICABLE.

CHORDS	SIZE	LUMBER DESCRIPTION
1- 3	2X 4	NO.1 KD15 SO. PINE
3- 5	2X 4	NO.1 KD15 SO. PINE
5- 1	2X 4	NO.2 KD15 SO. PINE

ALL WEBS 2X 4 NO.3 S.P.F.



MEMBER	FORCE (LBS)	CHORD SLOPE/12	MAXIMUM UNBRACED LENGTH (PLF)	W E B S FORCE (LBS)	CONC JT	LOAD LBS	CHORDS	SIZE	LUMBER DESCRIPTION	DESIGN CRITERIA
1-2	1157C	4-10-13	7.000	70.0	0-0	2-7	272C	2X 4	NO. 2 KD15 SO. PINE	TOP CH. L ₁ -25 PSF
2-3	1012C	4-5-3	7.000	70.0	0-0	3-7	410T	2X 4	NO. 2 KD15 SO. PINE	D ₁ -10 PSF
3-4	1012C	4-5-3	-7.000	70.0	0-0	3-6	410T	2X 4	NO. 2 KD15 SO. PINE	BOT CH. L ₁ -10 PSF
4-5	1157C	4-10-13	-7.000	70.0	0-0	4-6	272C	2X 4	NO. 3 S.P.F.	TOTAL LOAD=45 PSF
5-6	1000T	6-4-8	0.000	20.0	10.0					
6-7	679T	5-10-15	0.000	20.0	10.0					
7-1	1000T	6-4-8	0.000	20.0	10.0					

DESIGN SPECS. FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES, TP-1, 1995

MAX. PURLIN SPACE= 0.0 FT. MAX. UNBRACED BOT. CH. LEN.= 10.0 FT. *** PLYWOOD SHEATHING REQUIRED ON TOP CHORD ***

NOTE: LATERAL BRACES AND PURLINS INDICATED FOR TRUSS MEMBERS ARE REQUIRED TO REDUCE BUCKLING LENGTH OF MEMBER, AND SHOULD BE NAILED TO TRUSS MEMBERS WITH MINIMUM OF 2-1/2" COMMON WIRE NAILS. PROVISIONS MUST BE MADE AT ENDS OR SPECIFIED INTERVALS TO RESTRAIN OR ANCHOR LATERAL BRACING, BY OTHERS.

THIS TRUSS IS DESIGNED TO SUPPORT VERTICAL LOADS AS DETERMINED BY OTHERS AND SHOWN ON INPUT LISTING. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND BRACING OR OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED, IS THE RESPONSIBILITY OF THE PROJECT ARCHITECT OR ENGINEER.

ALL WEBS 2X 4

NO. 2 KD15 SO. PINE

NO. 2 KD15 SO. PINE

NO. 2 KD15 SO. PINE

NO. 3 S.P.F.

DESIGN CRITERIA

TOP CH. L₁-25 PSF

D₁-10 PSF

BOT CH. L₁-10 PSF

TOTAL LOAD=45 PSF

DESIGN CRITERIA

TOP CH. L₁-25 PSF

D₁-10 PSF

BOT CH. L₁-10 PSF

TOTAL LOAD=45 PSF

DESIGN CRITERIA

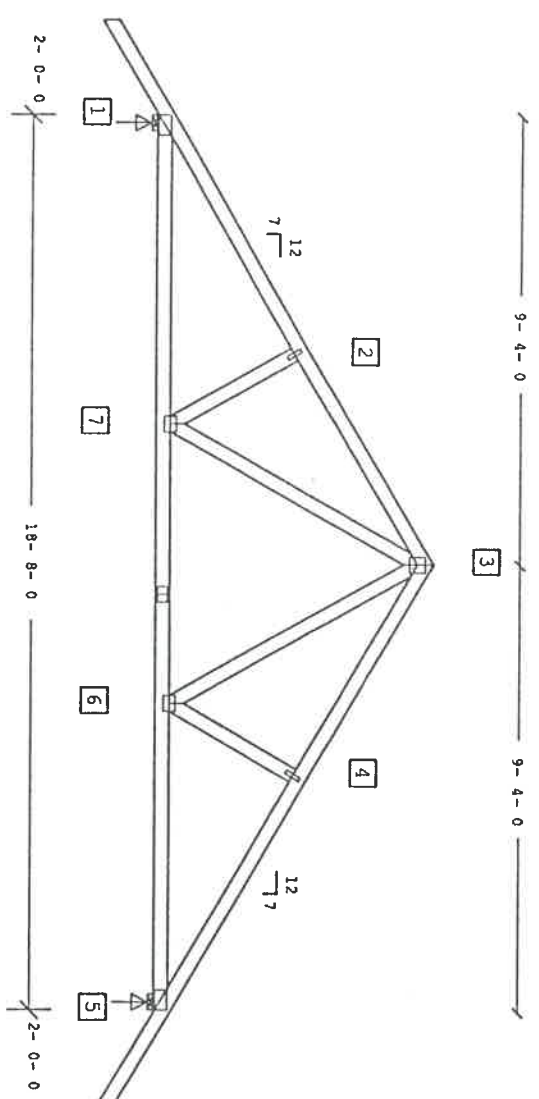
TOP CH. L₁-25 PSF

D₁-10 PSF

BOT CH. L₁-10 PSF

TOTAL LOAD=45 PSF

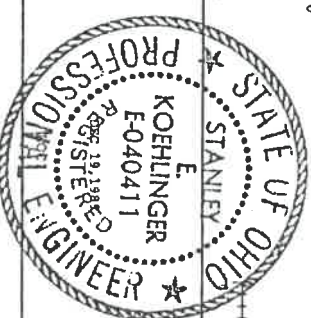
89-5163-4



GROSS BRG
 JT REACT IN-SX
 1 980 4-0
 5 980 4-0
 CAMBER= 0-0/8 HEEL = 4/16

Handling & Erection	Miscellaneous Information	Bracing Information	Connector Hardware	Lumber
<p>Carriers handling of components shall not be permitted. Temporary and permanent bracing for holding component joints and for resisting lateral forces shall be designed and installed by the fabricator. The use of blocking and fasteners are complete. At no time shall loads greater than design loads be applied to the component. Beams shall be installed in accordance with the connection be attached to install component at proper bearing points, right side up, and properly braced. Field of heavy bracing and contain any design assistance indicated. No responsibility shall be assumed for the design, fabrication and installation of components.</p>	<p>This data sheet and the information hereon is the property of Stark Truss Company, Inc. and is not to be copied in whole or in part or used for unauthorized duplication of the items disclosed herein or in any other way. The use of this component shall be specified by the designer of the complete structure. Obtain all necessary code compliance, approvals and instructions from the fabricator before using this component. If the design criteria listed above differs from the local building code requirements, DO NOT USE THIS DESIGN. Stark Truss Company, Inc. is not responsible for the design and construction of the structure. The designer of the structure is responsible for the design and construction of the structure. Stark Truss Company, Inc. is not responsible for the design and construction of the structure.</p>	<p>All bracing and bracing specified in this drawing shall be installed in accordance with the design and specifications of the fabricator. Web bracing where required shall be installed in accordance with the design and specifications of the fabricator. Chord members are assumed to be braced by the designer. Bracing shall be installed in accordance with the design and specifications of the fabricator. Bracing shall be installed in accordance with the design and specifications of the fabricator. Bracing shall be installed in accordance with the design and specifications of the fabricator.</p>	<p>Connectors are manufactured in accordance with the design and specifications of the fabricator. Plates must be fastened on both sides of the lumber with teeth fully embedded. Connectors shall be of the size, gauge and material specified in the design and specifications of the fabricator. Refer to the manufacturer's literature for details of joint types and dimensions. Dimensions are shown.</p>	<p>Lumber must have a grade stamp from a recognized lumber inspection bureau and must be of the size and species indicated in the design and specifications of the fabricator. Lumber shall be stored in a dry, well-ventilated area. Lumber shall be stored in a dry, well-ventilated area. Lumber shall be stored in a dry, well-ventilated area. Lumber shall be stored in a dry, well-ventilated area.</p>

IMPORTANT: READ ALL NOTES ON THIS DRAWING!



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11
DEC 19, 1989 F3-19F

WARNING- VERIFY YOUR INPUT TO AVOID DESIGN AND FABRICATION MISTAKES.
YOU ARE SOLELY RESPONSIBLE FOR ERRORS RESULTING FROM WRONG INPUT

REQUEST NO. CALCONLY QUOTE WAUSEON WOODWORKING (545624) TG
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ALL RIGHTS RESERVED. AUTOTRUS

13 7
REQUEST NO. CALCONLY QUOTE WAUSEON WOODWORKING (545624) TG T-4
DEC 19, 1989 F3-19F

SPAN (OUT TO OUT) 18.667
NO. OF JOINTS 7
LOC. OF REACTIONS 1 5
SHORT TERM INCREASE TCH 1.15 BCH 1.15 WEBS 1.15 PLATES 1.15

TOP CHORD LOAD SHARE = 15 BOTTOM CHORD LOAD SHARE = 15
PLYWOOD SHEATHING USED ON TOP CHORD WHERE APPLICABLE.

TRUSS GEOMETRY AND LOAD DATA	VER. DISP.	VERTICAL UNIF. ID.	VERTICAL CONC. ID.
FT	SLOPE/12	PLF	LBS
1	4.8988	-70.00	-140.00
2	4.4345	-70.00	0.00
3	4.4345	-70.00	0.00
4	4.8988	-70.00	0.00
5	-6.3770	-20.00	-140.00
6	-5.9127	0.0000	0.00
7	-6.3770	0.0000	0.00

TOTAL POSITIVE DISPLACEMENT= 18.67

NO. OF WEBS= 4
2- 7 3- 7 3- 6 4- 6

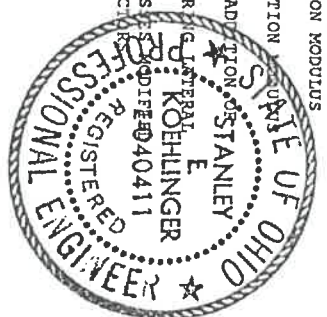
GROSS REACTIONS (LBS):
RV- 1= 980.0 RV- 5= 980.0
RH- 1= 0.0

MEM	FORCE LBS	WID IN.	DEP IN.	FB PSI	FC:FT Q	P/AF	VM/ZF	HM/ZF	CSI	LAT. BRC
TOP CHORD MEMBERS										
1- 2	-1157.	1.50	3.50	2013	1323	0.90	0.17	0.27	0.00	0.44
2- 3	-1012.	1.50	3.50	2013	1323	0.90	0.15	0.27	0.00	0.42
3- 4	-1012.	1.50	3.50	2013	1323	0.90	0.15	0.27	0.00	0.42
4- 5	-1157.	1.50	3.50	2013	1323	0.90	0.17	0.27	0.00	0.44
BOT CHORD MEMBERS										
5- 6	1000.	1.50	3.50	2013	1035	1.00	0.18	0.17	0.00	0.36
6- 7	679.	1.50	3.50	2013	1035	1.00	0.12	0.17	0.00	0.30
7- 1	1000.	1.50	3.50	2013	1035	1.00	0.18	0.17	0.00	0.36
WEB MEMBERS										
2- 7	-272.	1.50	3.50	633	465	0.00	0.11	0.00	0.00	0.11
3- 7	410.	1.50	3.50	633	374	0.00	0.21	0.00	0.00	0.21

CHORDS	SIZE	LUMBER DESCRIPTION
1- 3	2X 4	NO.2 KD15 SO. PINE
3- 5	2X 4	NO.2 KD15 SO. PINE
5- 1	2X 4	NO.2 KD15 SO. PINE
ALL WEBS	2X 4	NO.3 S.P.F.

DEFLECTION AT 6 = -0.0600 INCHES
DEFLECTION BETWEEN 6- 7 = -0.0959 INCHES

EXPLANATIONS:
P/AF = AXIAL FORCE DIVIDED BY THE CROSS-SECTIONAL AREA AND THE ALLOWABLE UNIT STRESS IN TENSION OR COMPRESSION.
VM/ZF= VERTICAL BENDING MOMENT DIVIDED BY THE SECTION MODULUS AND THE ALLOWABLE UNIT STRESS IN BENDING.
HM/ZF= HORIZONTAL BENDING MOMENT DIVIDED BY THE SECTION MODULUS AND THE ALLOWABLE UNIT STRESS IN BENDING.
CSI = COMBINED STRESS INTERACTION EQUATION IS THE ADDITION OF P/AF + VM/ZF + HM/ZF
LAT. = MAXIMUM DISTANCE ALLOWABLE (FT) WITHOUT REQUIRING SUPPORT.
STRESSES SHOWN (FB FT FC) ARE ALLOWABLE LUMBER STRESSES BY THE SHORT TERM INCREASE AND OTHER APPROPRIATE FACTORS WHEREVER APPLICABLE.



MEMBER	FORCE (LBS)	HOR DISP	VERT DISP	DEPTH IN	LOAD (LBS)	MAXIMUM MEMBER LENGTH	W E B S FORCE (LBS)	CONC JT	LOAD LBS	CHORDS	SIZE	LONGER	DESCRIPTION	DESIGN CRITERIA
1-2	2895C	5-3-5	3.827	70.0	0.0	2-9	387C	1	140	1-4	2X 4	NO.2 KD15 SO. PINE	TOP CH. LL= 25 PSF	
2-3	2895C	5-3-2	3.827	70.0	0.0	3-9	730T	1	140	4-6	2X 4	NO.2 KD15 SO. PINE	DL= 10 PSF	
3-4	1763C	5-2-2	3.827	70.0	0.0	3-8	653C	6	140	6-1	2X 4	NO.2 KD15 SO. PINE	BOF CH. LL= 0 PSF	
4-5	1703C	4-5-7	-7.000	70.0	0.0	4-8	802T	6	140	2X 4	NO.2 KD15 SO. PINE	DL= 10 PSF		
5-6	1850C	4-11-0	-7.000	70.0	0.0	4-7	419T	6	140	NO.3 S.P.F.	NO.3 S.P.F.	TOTAL LOAD= 45 PSF		
6-7	1598T	6-4-13	0.000	20.0	10.0	5-7	276C	6	140	NO.3 S.P.F.	NO.3 S.P.F.	TOTAL LOAD= 45 PSF		
7-8	1271T	6-2-11	0.000	20.0	10.0	5-7	276C	6	140	NO.3 S.P.F.	NO.3 S.P.F.	TOTAL LOAD= 45 PSF		
8-9	2010T	6-6-3	0.000	20.0	10.0	5-7	276C	6	140	NO.3 S.P.F.	NO.3 S.P.F.	TOTAL LOAD= 45 PSF		
9-1	2748T	7-4-6	0.000	20.0	10.0	5-7	276C	6	140	NO.3 S.P.F.	NO.3 S.P.F.	TOTAL LOAD= 45 PSF		

DESIGN SPECS. FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES, TPI, 1985

NOTE: LATERAL BRACES AND PURLINS INDICATED FOR TRUSS MEMBERS ARE REQUIRED TO REDUCE BUCKLING LENGTH OF MEMBERS, AND SHOULD BE NAILED TO TRUSS MEMBERS WITH MINIMUM OF 2-1/8" COMMON WIRE NAILS. PROVISIONS MUST BE MADE AT ENDS OR SPECIFIED INTERVALS TO RESTRAIN OR ANCHOR LATERAL BRACING, BY OTHERS.

MAX. PURLIN SPACING = 0.0 FT. / MAX. TOP CHORD BOT. CH. LEN. = 10.0 FT.

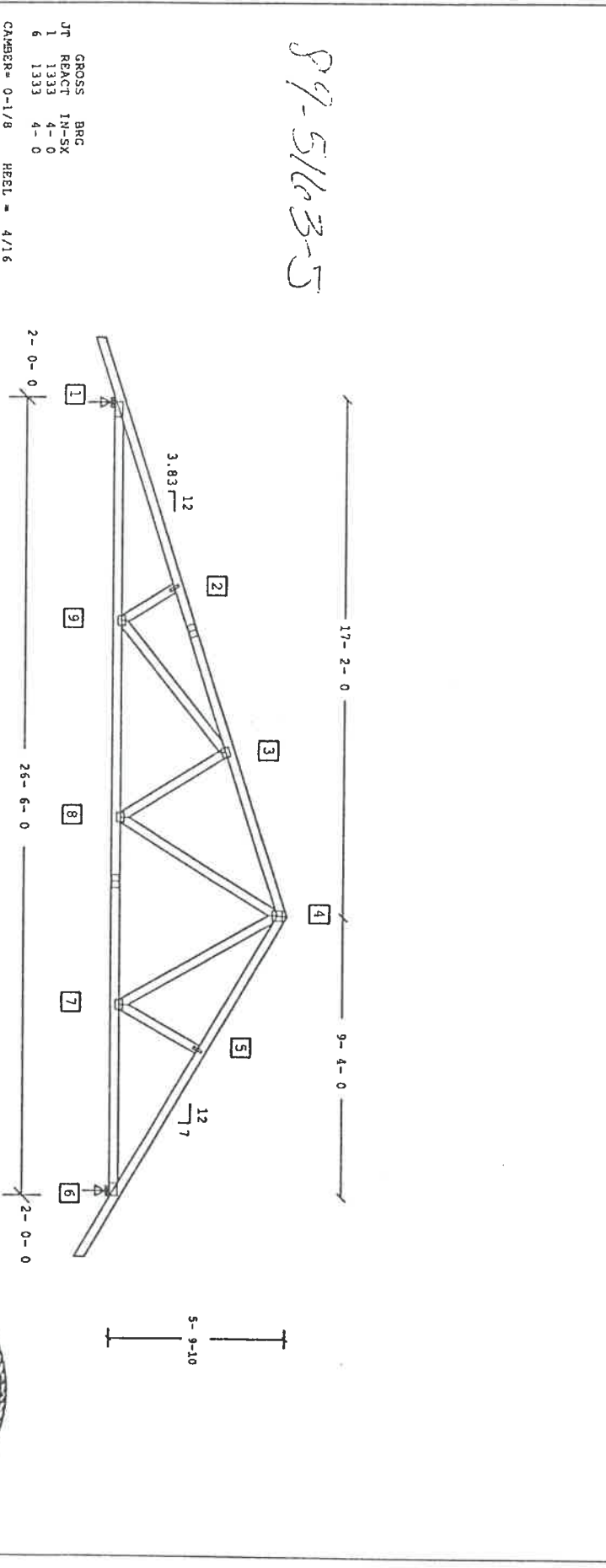
*** PLYWOOD SHEATHING REQUIRED ON TOP CHORD ***

THIS TRUSS IS DESIGNED TO SUPPORT VERTICAL LOADS AS DETERMINED BY OTHERS AND SHOWN ON INPUT LISTING. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, DRAWING METHODS, WIND BRACING OR OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED, IS THE RESPONSIBILITY OF THE PROJECT ARCHITECT OR ENGINEER.

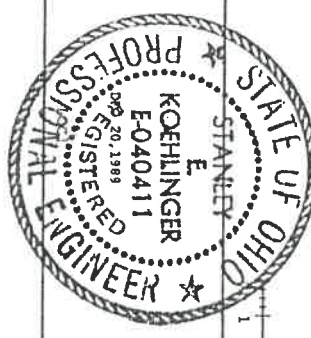
INCREASES PER CENT
LUMBER = 15
NAIL = 15
ICH LS = 15 BCH LS = 15

NAIL VALUES (PSI) GROSS
CHORDS MBS
MAX MIN MAX MIN
GNO20 228 180 190 140

LEFT OVERHANG = 2-0-0
RIGHT OVERHANG = 2-0-0



Handling & Erection	Miscellaneous Information	Bracing Information	Connector Hardware	Lumber
<p>Caution handling of components shall not be permitted. Fasteners and fastening materials shall be specified and isolated by others. No loads are to be applied to the component until the bracing and fastenings are complete. The use of this component shall be specified by the designer of the complete structure.</p> <p>Care must be exercised to install components at proper bearing points, snug tight, and properly braced. Read all instructions and drawings carefully. Gang-Nail Systems, Inc. is not responsible for the fabrication, handling, shipment and installation of components.</p>	<p>This data sheet and the information herein is the property of Gang-Nail Systems, Inc. and is not to be copied in whole or in part or used for any other purpose without the written consent of Gang-Nail Systems, Inc. The use of this component shall be specified by the designer of the complete structure.</p> <p>The designer of the complete structure shall be responsible for providing the designer of the truss with the necessary information to design the truss members and bracing. Gang-Nail Systems, Inc. is not responsible for the design of the truss members and bracing.</p>	<p>All lateral bracing specified is for bracing individual web members and must be installed in accordance with the design of the complete structure. Gang-Nail Systems, Inc. is not responsible for the design of the complete structure.</p>	<p>Connector plates are manufactured in accordance with TPI. Plates must be installed on both faces of the truss members and must be of the size, gauge and capacity shown. Anchor bolts are shown in the drawing and must be installed in accordance with the design of the complete structure.</p>	<p>Lumber must bear a grade mark from an approved inspection bureau and must be of the size, gauge and capacity shown. Lumber shall be cut to the length shown and equal to or better than the grade specified.</p>



IMPORTANT: READ ALL NOTES ON THIS DRAWING!

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0.42
0.13
0.41

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DEC 20, 1989 F3-19F

WARNING- VERIFY YOUR INPUT TO AVOID DESIGN AND FABRICATION MISTAKES.
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REQUEST NO. CALCONLY QUOTE MAUSEON WOODWORKING (545624) TG

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13 7
REQUEST NO. CALCONLY QUOTE MAUSEON WOODWORKING (545624) TG T-5
DEC 20, 1989 F3-19F

SPAN (OUT TO OUT) 26.500
NO. OF JOINTS 9
LOC. OF REACTIONS 1 6
SHORT TERM INCREASE TCH 1.15 BCH 1.15 WEBS 1.15 PLATES 1.15

TOP CHORD LOAD SHARE = 15 BOTTOM CHORD LOAD SHARE = 15

PLYWOOD SHEATHING USED ON TOP CHORD WHERE APPLICABLE.

TRUSS GEOMETRY AND LOAD DATA

JT	HOR. DISP.	VER. DISP.	VERTICAL UNIF. LD.	VERTICAL CONC. LD.
FT	FT	SLOPE/12	PLF	IBS
1	6.2768	3.8275	-70.00	-140.00
2	5.4277	3.8275	-70.00	0.00
3	5.4277	3.8275	-70.00	0.00
4	4.4517	-7.0000	-70.00	0.00
5	4.9160	-7.0000	-70.00	0.00
6	-6.3999	0.0000	-20.00	-140.00
7	-6.2244	0.0000	-20.00	0.00
8	-6.5133	0.0000	-20.00	0.00
9	-7.3624	0.0000	-20.00	0.00

TOTAL POSITIVE DISPLACEMENT= 26.50

NO. OF WEBS= 6
2- 9 3- 9 3- 8 4- 8 4- 7 5- 7

GROSS REACTIONS(LBS):
RV- 1= 1332.5 RV- 6= 1332.5
RH- 1= 0.0

MEM FORCE MID DEP FB FC:FT Q P/AF VM/ZF HM/ZF CSI LAT. BRC

MEM	LBS	IN.	IN.	PSI	PSI	Q	P/AF	VM/ZF	HM/ZF	CSI	LAT.	BRC
TOP CHORD MEMBERS												
1- 2	-2885.	1.50	3.50	1810	1245	0.80	0.44	0.38	0.00	0.82	0.0	
2- 3	-2691.	1.50	3.50	1824	1245	0.80	0.41	0.38	0.00	0.79	0.0	
3- 4	-1765.	1.50	3.50	2013	1323	0.85	0.25	0.36	0.00	0.62	0.0	
4- 5	-1703.	1.50	3.50	2013	1323	0.90	0.25	0.27	0.00	0.52	0.0	
5- 6	-1850.	1.50	3.50	2013	1323	0.90	0.27	0.27	0.00	0.54	0.0	
BOT CHORD MEMBERS												
6- 7	1598.	1.50	3.50	2013	1035	1.00	0.29	0.18	0.00	0.48	10.0	
7- 8	1271.	1.50	3.50	2013	1035	1.00	0.23	0.20	0.00	0.43	10.0	
8- 9	2010.	1.50	3.50	2013	1035	1.00	0.37	0.22	0.00	0.59	10.0	

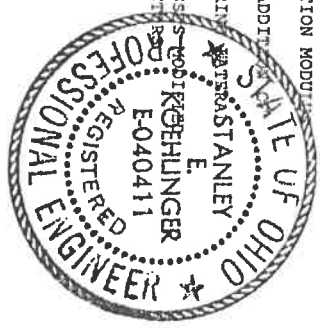
MEMBERS	2748.	1.50	3.50	2013	1035	1.00	0.51	0.22	0.00	0.73	10.0
2- 9	-387.	1.50	3.50	633	483	0.00	0.15	0.00	0.00	0.15	2.3
3- 9	730.	1.50	3.50	633	374	0.00	0.37	0.00	0.00	0.37	5.7
3- 8	-653.	1.50	3.50	633	411	0.00	0.30	0.00	0.00	0.30	4.3
4- 8	802.	1.50	3.50	633	374	0.00	0.41	0.00	0.00	0.41	6.4
4- 7	419.	1.50	3.50	633	374	0.00	0.21	0.00	0.00	0.21	6.2
5- 7	-276.	1.50	3.50	633	464	0.00	0.11	0.00	0.00	0.11	3.2

DEFLECTION AT 9 = -0.2704 INCHES
DEFLECTION BETWEEN 8- 9 = -0.3080 INCHES

EXPLANATIONS:

P/AF = AXIAL FORCE DIVIDED BY THE CROSS-SECTIONAL AREA AND THE ALLOWABLE UNIT STRESS IN TENSION OR COMPRESSION.
VM/ZF= VERTICAL BENDING MOMENT DIVIDED BY THE SECTION MODULUS AND THE ALLOWABLE UNIT STRESS IN BENDING.
HM/ZF= HORIZONTAL BENDING MOMENT DIVIDED BY THE SECTION MODULUS AND THE ALLOWABLE UNIT STRESS IN BENDING.
CSI = COMBINED STRESS INTERACTION EQUATION IS THE ADDITIONAL P/AF + VM/ZF + HM/ZF
LAT. = MAXIMUM DISTANCE ALLOWABLE (FT) WITHOUT REQUIRING BRG SUPPORT.
STRESSES SHOWN (PB FT FC) ARE ALLOWABLE LUMBER STRESSES MODIFIED BY THE SHORT TERM INCREASE AND OTHER APPROPRIATE FACTORS WHEREVER APPLICABLE.

CHORDS	SIZE	LUMBER DESCRIPTION
1- 4	2X 4	NO.2 KD15 SO. PINE
4- 6	2X 4	NO.2 KD15 SO. PINE
6- 1	2X 4	NO.2 KD15 SO. PINE
ALL WEBS	2X 4	NO.3 S.P.F.



FLOOR FRAMING DICK

GIVE ME INFO ON DIM. DOOR
LAMB BEAM

128.01'

35

93.01'

62.8'

30.11'

12

CASEMINZI (ANDERSON)

61
528
8-4"

$$\begin{array}{r} 32-9'' \\ 1-4 \\ \hline 31-5'' = \textcircled{15-9''} \end{array}$$

(40x10)
 $15-9'' \times 50'' = 788 \#$

$$M = \frac{W \times L}{8} = \frac{788 \times \overset{13}{13-4''} \times \overset{12}{13-9''} \times 12}{8} = \frac{170,208}{8} = 210,210$$

$$S = \frac{M}{S} = \frac{121}{170+15}$$

$$S = \frac{6d^2}{6} = 94.9 \quad 4-2 \times 12 \# 254P$$

- 10-2
- 13-4
- 12-4
- 7-10

$$4 \overline{) 438} \quad 10-11''$$

$$\begin{array}{r} 16-5 \\ 7 \\ \hline 23-5'' = \textcircled{11-9''} \times 50 = 588 \end{array}$$

MEMBER	FORCE (LBS)	CHORDS	SIZE	LUMBER DESCRIPTION	DESIGN CRITERIA
1 HI.01 GNO20 3.0X10.0	4171C	1-4	2X 4	NO.2 KD15 SO. PINE	TOP CH. LL= 25 PSF
2 IN.01 GNO20 1.0X 4.0	3046C	4-7	2X 4	NO.2 KD15 SO. PINE	DI= 10 PSF
3 IN.02 GNO20 3.0X 4.0	3046C	7-1	2X 4	NO.1 D.KD15 SO.PINE	BOT CH. LL= 20 PSF
4 PR.02 GNO20 4.0X 5.0	4028C				TOTAL LOAD= 65 PSF
5 IN.02 GNO20 3.0X 4.0	4028C				
6 IN.01 GNO20 1.0X 4.0	4171C				
7 HI.01 GNO20 3.0X10.0	4171C				
8 IN.02 GNO20 3.0X 4.0	3850T				
9 IN.02 GNO20 3.0X 5.0	3103T				
10 IN.02 GNO20 3.0X 5.0	3103T				
11 IN.02 GNO20 3.0X 4.0	3850T				

DESIGN SPCS. FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES, TPI, 1985

NOTE: LATERAL BRACES AND PURLINS INDICATED FOR TRUSS MEMBERS BE NAILIED TO TRUSS MEMBERS WITH MINIMUM OF 2-10D COMMON WIRE NAILS. PROVISIONS MUST BE MADE AT ENDS OR SPECIFIED INTERVALS TO RESTRAIN OR ANCHOR LATERAL BRACING. BY OTHERS.

MAX. PURLIN SPACE= 0.0 FT. * MAX. UNBRACED BOT. CH. LEN.= 10.0 FT. *** PLYWOOD SHEATHING REQUIRED ON TOP CHORD ***

THIS TRUSS IS DESIGNED TO SUPPORT VERTICAL LOADS AS DETERMINED BY OTHERS AND SHOWN ON INPUT LISTING. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND BRACING OR OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED, IS THE RESPONSIBILITY OF THE PROJECT ARCHITECT OR ENGINEER.

MAIL VALUES (PST) GROSS CHORDS WEBS

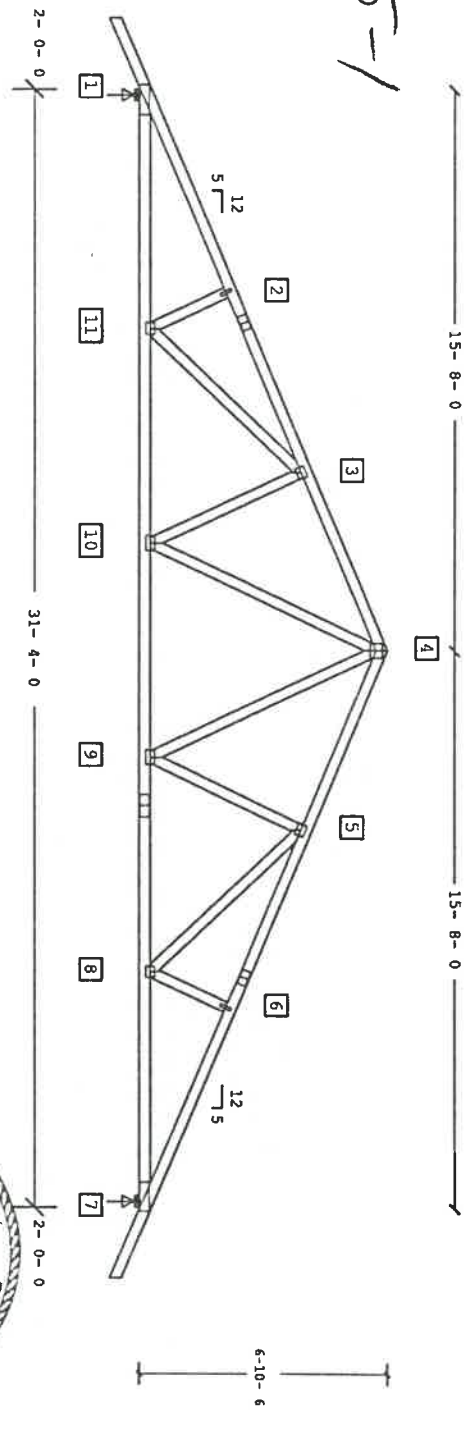
MAX MIN MAX MIN

GNO20 228 180 190 140

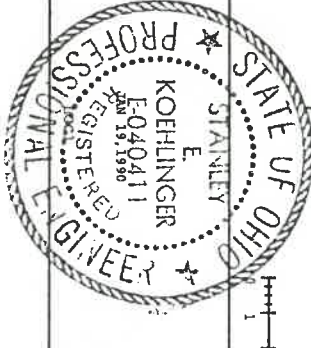
GNA16 163 150 122 119

LEFT OVERHANG= 2-0-0

RIGHT OVERHANG= 2-0-0



Handling & Erection	Miscellaneous Information	Bracing Information	Connector Hardware	Lumber
Caution handling of components shall not be permitted. Temporary and permanent bracing for loading component must be installed by others. No load is to be applied to the component until after all bracing and fastenings are complete. All in-line web loads greater than design loads be applied to the web. Care must be exercised to lateral component at proper bearing points, tight side up, and properly braced. Field all members. Do not use any design assumptions indicated on drawings. Do not use any design assumptions indicated on drawings for the fabrication, handling, shipment and installation of components.	This data sheet and the information herein is the property of Stark Truss Company, Inc. and is not to be copied in whole or in part or used for any other purpose without the written consent of Stark Truss Company, Inc. The use of this component shall be specified by the designer of the complete structure. Design codes, approvals and instructions from the designer of the complete structure before using this component. Requirements: DO NOT USE THIS DESIGN. Stark Truss Company, Inc. is not responsible for the design of the unit shown on the basis of data provided by the customer and shown on this drawing.	All lateral bracing specified is for individual web members and must be equally spaced along web length. Lateral bracing members are assumed to be equally spaced along web length. Restraint of lateral bracing and purlins for fielding by the designer of the complete structure.	Connector plates are manufactured in accordance with TPI. Plates must be identified on both ends of the line and identified on opposite ends of the line. Any TPI fasteners must be of the size, gauge and capacity shown. The use of any other fasteners for delineation of joint types and locations. Position of plates and fasteners must be shown on the drawing.	Lumber must bear a grade mark of the lumber and species shown and equal to or better than the grade specified. Design Criteria: This drawing and the material specified are in accordance with the latest revision of NDS, ATC and TPI.



IMPORTANT: READ ALL NOTES ON THIS DRAWING!

GA
0.91
0.91
0.54

11
JAN 19, 1990 F3-19F

WARNING- VERIFY YOUR INPUT TO AVOID DESIGN AND FABRICATION MISTAKES.
YOU ARE SOLELY RESPONSIBLE FOR ERRORS RESULTING FROM WRONG INPUT

REQUEST NO. CALCONLY QUOTE MAUSEON WOODWORKING (545624) TG
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13 7
REQUEST NO. CALCONLY QUOTE MAUSEON WOODWORKING (545624) TG T-2
JAN 19, 1990 F3-19F

SPAN (OUT TO OUT) 31.333
NO. OF JOINTS 11
LOC. OF REACTONS 1 7
SHORT TERM INCREASE TCH 1.15 BCH 1.15 WEBS 1.15 PLATES 1.15
TOP CHORD LOAD SHARE = 15 BOTTOM CHORD LOAD SHARE = 15

PLYWOOD SHEATHING USED ON TOP CHORD WHERE APPLICABLE.

TRUSS GEOMETRY AND LOAD DATA	VER. DISP.	UNIF. LD.	VERTICAL CONC. LD.
JT HOR. DISP.	FT	SLOPE/12	PLF
1	5.6556	5.0000	-70.00
2	5.0056	5.0000	-70.00
3	5.0056	5.0000	-70.00
4	5.0056	5.0000	-70.00
5	5.0056	5.0000	-70.00
6	5.6556	5.0000	-70.00
7	-6.6567	0.0000	-60.00
8	-6.0067	0.0000	-60.00
9	-6.0067	0.0000	-60.00
10	-6.0067	0.0000	-60.00
11	-6.6567	0.0000	-60.00

TOTAL POSITIVE DISPLACEMENT= 31.33

NO. OF WEBS= 8
2-11 3-11 3-10 4-10 4-9 5-9 5-8 6-8

GROSS REACTIONS(LBS):
RV- 1= 2176.7 RV- 7= 2176.7
RH- 1= 0.0

MEM FORCE MID DEP FB FC:FT Q P/AF VM/2F HM/2F CSI LAT.
LBS IN. IN. PSI PSI

MEM	FORCE	MID	DEP	FB	FC:FT	Q	P/AF	VM/2F	HM/2F	CSI	LAT.
	LBS	IN.	IN.	PSI	PSI					BRC	
1-2	-4171.	1.50	3.50	2013	1323	0.85	0.60	0.31	0.00	0.91	0.0
2-3	-4028.	1.50	3.50	2013	1323	0.85	0.58	0.31	0.00	0.89	0.0
3-4	-3046.	1.50	3.50	2013	1323	0.85	0.44	0.31	0.00	0.75	0.0
4-5	-3046.	1.50	3.50	2013	1323	0.85	0.44	0.31	0.00	0.75	0.0
5-6	-4028.	1.50	3.50	2013	1323	0.85	0.58	0.31	0.00	0.89	0.0
6-7	-4171.	1.50	3.50	2013	1323	0.85	0.60	0.31	0.00	0.91	0.0

BOT CHORD MEMBERS

7-8	3850.	1.50	3.50	2818	1438	1.00	0.51	0.40	0.00	0.91	10.0
8-9	3103.	1.50	3.50	2818	1438	1.00	0.41	0.40	0.00	0.81	10.0
9-10	2351.	1.50	3.50	2818	1438	1.00	0.31	0.38	0.00	0.69	10.0
10-11	3103.	1.50	3.50	2818	1438	1.00	0.41	0.40	0.00	0.81	10.0
11-1	3850.	1.50	3.50	2818	1438	1.00	0.51	0.40	0.00	0.91	10.0

DEFLECTION AT 9 = -0.4069 INCHES
DEFLECTION BETWEEN 9-10 = -0.5036 INCHES

EXPLANATIONS:
P/AF = AXIAL FORCE DIVIDED BY THE CROSS-SECTIONAL AREA AND THE ALLOWABLE UNIT STRESS IN TENSION OR COMPRESSION.
VM/2F = VERTICAL BENDING MOMENT DIVIDED BY THE SECTION MODULUS AND THE ALLOWABLE UNIT STRESS IN BENDING.
HM/2F = HORIZONTAL BENDING MOMENT DIVIDED BY THE SECTION MODULUS AND THE ALLOWABLE UNIT STRESS IN BENDING.
CSI = COMBINED STRESS INTERACTION EQUATION IS THE ADDITION STANLEY P/AF + VM/2F + HM/2F
LAT. = MAXIMUM DISTANCE ALLOWABLE (FT) WITHOUT REQUIRE LUMBER SUPPORT.
STRESSES SHOWN (FB FT FC) ARE ALLOWABLE LUMBER STRESSES BY THE SHORT TERM INCREASE AND OTHER APPROPRIATE FACTORS WHEREVER APPLICABLE.

CHORDS	SIZE	LUMBER DESCRIPTION
1-4	2X 4	NO.2 KD15 SO. PINE
4-7	2X 4	NO.2 KD15 SO. PINE
7-1	2X 4	NO.1 D.KD15 SO.PINE

ALL WEBS 2X 4 NO.3 S.P.F.



MEMBER	FORCE (LBS)	CHORDS	MEMBER FORCE (LBS)	CONC LOAD	CHORDS	SIZE	LUMBER DESCRIPTION	DESIGN CRITERIA
1 H101 GNO20 3.0X10.0	4171C	5-7-14	70.0	2-11	1-4	2X 4	NO.2 KD15 SO. PINE	TOP CH. LL= 25 PSF
2 IN11 GNO20 1.0X 4.0	4028C	5-0-1	5.000	0.0	4-7	2X 4	NO.2 KD15 SO. PINE	DL= 10 PSF
3 IN02 GNO20 3.0X 4.0	3046C	5-0-1	5.000	0.0	7-1	2X 4	NO.1 D.KD15 SO.PINE	BOT CH. DL= 20 PSF
4 PR12 GNO20 4.0X 5.0	3046C	5-0-1	5.000	0.0	2X 4			TOTAL LOAD= 65 PSF
5 IN02 GNO20 3.0X 4.0	4028C	5-0-1	5.000	0.0				
6 IN11 GNO20 1.0X 4.0	4171C	5-7-14	70.0	0.0				
7 H101 GNO20 3.0X 4.0	4171C	5-7-14	70.0	0.0				
8 IN02 GNO20 3.0X 5.0	3850T	6-7-14	60.0	10.0				
9 IN02 GNO20 3.0X 5.0	3103T	6-0-1	0.000	10.0				
10 IN02 GNO20 3.0X 4.0	2351T	6-0-1	0.000	10.0				
11 IN02 GNO20 3.0X 4.0	3103T	6-0-1	0.000	10.0				
12 SPLICERS	3850T	6-7-14	60.0	10.0				
2-3 SP10 GNO20 3.0X 5.0								
5-6 SP10 GNO20 3.0X 5.0								
8-9 SP10 GNA16 3.0X 7.5								

MAX. PURLIN SPACE= 0.0 FT. MAX. UNBRACED BOT. CH. LEN.= 10.0 FT.
 *** PLYWOOD SHEATHING REQUIRED ON TOP CHORD ***

NOTE: LATERAL BRACES AND PURLINS INDICATED FOR TRUSS MEMBERS ARE REQUIRED TO REDUCE BUCKLING LENGTH OF MEMBER, AND SHOULD BE NAILED TO TRUSS MEMBERS WITH MINIMUM OF 2-10D COMMON WIRE NAILS. PROVISIONS MUST BE MADE AT ENDS OR SPECIFIED INTERVALS TO RESTRAIN OR ANCHOR LATERAL BRACING. BY OTHERS.

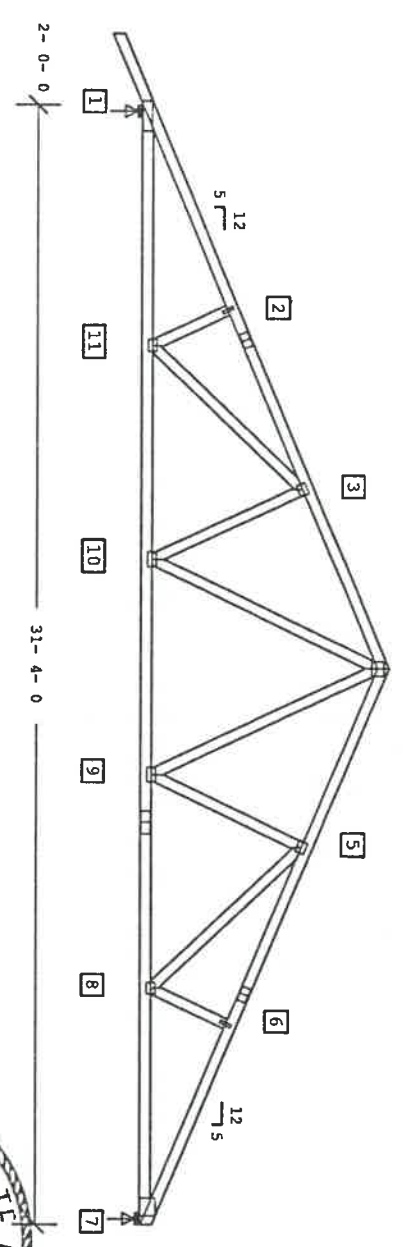
THIS TRUSS IS DESIGNED TO SUPPORT VERTICAL LOADS AS DETERMINED BY OTHERS AND SHOWN ON INPUT LISTING. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND BRACING OR OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED, IS THE RESPONSIBILITY OF THE PROJECT ARCHITECT OR ENGINEER.

ALL WEBS 2X 4
 NO.3 S.P.F.
 INPUT DEF. L/360
 INCREASES (PER CENT)
 LUMBER= 15 NAIL= 15
 TCH LS= 15 BCH LS= 15

NAIL VALUES (PST) GROSS
 CHORDS WEBS
 MAX MIN MAX MIN
 GNO20 228 180 190 140
 GNA16 163 150 122 119

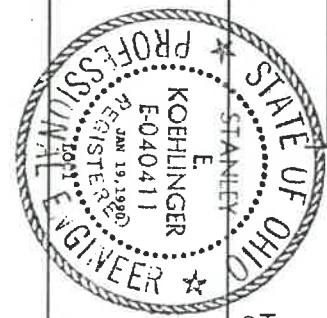
LEFT OVERHANG = 2-0-0

90-5005-2
 Revised



Handling & Erection	Miscellaneous Information	Bracing Information	Connector Hardware	Lumber
<p>Carries handling of components shall not be permitted. Lumber must be stored in a dry, well-ventilated area and protected from moisture. Lumber shall be stored in a dry, well-ventilated area and protected from moisture. Lumber shall be stored in a dry, well-ventilated area and protected from moisture.</p>	<p>This data sheet and the information herein is the property of Stark Truss Company and is not to be copied in whole or in part or used for any other purpose without the written consent of Stark Truss Company. The use of this component shall be specified by the designer of the structure. The designer of the structure shall be responsible for the design of the complete structure before using this component. When this drawing is signed and sealed, Stark Truss Company, Inc. is not responsible for the structural design of the work shown on this drawing.</p>	<p>All lateral bracing specified is for bracing members with members and end joints. Bracing members shall be equally spaced along web length. Diagonal members are assumed to be braced. Lateral bracing and purlins or ceiling members shall be braced. Lateral bracing and purlins or ceiling members shall be braced. Lateral bracing and purlins or ceiling members shall be braced.</p>	<p>Connector plates are manufactured in accordance with TP1. Connector plates shall be installed on both faces of the lumber with teeth fully embedded. Plates must be of the size, gauge and finish shown. Also, final hole detail shall be as shown. Also, final hole detail shall be as shown. Also, final hole detail shall be as shown.</p>	<p>Lumber must bear a grade mark from an approved lumber grading agency and must be of the species and grade shown and equal to or better than the grade specified. Design Criteria: The design and the material specified are in accordance with the latest edition of NDS, ATC and TP1.</p>

IMPORTANT: READ ALL NOTES ON THIS DRAWING!



AA
 0.91
 0.31
 0.36

GROSS BRG	REACT IN-SX
1	2177
7	2037

CAMBER= 0-1/8 HEEL = 4/16

11
JAN 19, 1990 F3-19F

WARNING- VERIFY YOUR INPUT TO AVOID DESIGN AND FABRICATION MISTAKES.
YOU ARE SOLELY RESPONSIBLE FOR ERRORS RESULTING FROM WRONG INPUT

REQUEST NO. CALCONLY QUOTE WAUSEBON WOODWORKING (545624) TG
GANG NATL SYSTEMS INC. COPYRIGHT 1988.
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13 7
REQUEST NO. CALCONLY QUOTE WAUSEBON WOODWORKING (545624) TG T-3
JAN 19, 1990 F3-19F

SPAN (OUT TO OUT) 31.333
NO. OF JOINTS 11
LOC. OF REACTIONS 1 7
SHORT TERM INCREASE TCH 1.15 BCH 1.15 WEBS 1.15 PLATES 1.15

TOP CHORD LOAD SHARE = 15 BOTTOM CHORD LOAD SHARE = 15
PLYWOOD SHEATHING USED ON TOP CHORD WHERE APPLICABLE.

TRUSS GEOMETRY AND LOAD DATA	VER. DISP.	VERTICAL UNIF. ID.	VERTICAL PLF	VERTICAL CONC. ID.	VERTICAL LBS
JT HOR. DISP. FT	SLOPE/12				
1	5.6536	5.0000	-70.00	-140.00	0.00
2	5.0056	5.0000	-70.00	0.00	0.00
3	5.0056	5.0000	-70.00	0.00	0.00
4	5.0056	-5.0000	-70.00	0.00	0.00
5	5.0056	-5.0000	-70.00	0.00	0.00
6	5.6556	-5.0000	-70.00	0.00	0.00
7	-6.6567	0.0000	-60.00	0.00	0.00
8	-6.0067	0.0000	-60.00	0.00	0.00
9	-6.0067	0.0000	-60.00	0.00	0.00
10	-6.0067	0.0000	-60.00	0.00	0.00
11	-6.6567	0.0000	-60.00	0.00	0.00

TOTAL POSITIVE DISPLACEMENT= 31.33

NO. OF WEBS= 8
2-11 3-11 3-10 4-10 4-9 5-9 5-8 6-8

GROSS REACTIONS(LBS):
RV-1= 2176.7 RV-7= 2036.7
RH-1= 0.0

MEM FORCE MID DEP FB FC:FT Q P/AF VM/ZF HM/ZF CSI LAT.
LBS IN. IN. PSI PSI

TOP CHORD MEMBERS

1-2	-4171.	1.50	3.50	2013	1323	0.85	0.60	0.31	0.00	0.91	0.0
2-3	-4028.	1.50	3.50	2013	1323	0.85	0.58	0.31	0.00	0.89	0.0
3-4	-3046.	1.50	3.50	2013	1323	0.85	0.44	0.31	0.00	0.75	0.0
4-5	-4028.	1.50	3.50	2013	1323	0.85	0.44	0.31	0.00	0.75	0.0
5-6	-4028.	1.50	3.50	2013	1323	0.85	0.58	0.31	0.00	0.89	0.0
6-7	-4171.	1.50	3.50	2013	1323	0.85	0.60	0.31	0.00	0.91	0.0

BOT CHORD MEMBERS

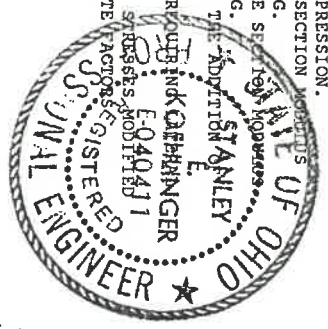
7-8	3850.	1.50	3.50	2818	1438	1.00	0.51	0.40	0.00	0.91	10.0
8-9	3103.	1.50	3.50	2818	1438	1.00	0.41	0.40	0.00	0.81	10.0
9-10	2351.	1.50	3.50	2818	1438	1.00	0.31	0.38	0.00	0.69	10.0
10-11	3103.	1.50	3.50	2818	1438	1.00	0.41	0.40	0.00	0.81	10.0
11-1	3850.	1.50	3.50	2818	1438	1.00	0.51	0.40	0.00	0.91	10.0
2-11	-337.	1.50	3.50	633	479	0.00	0.13	0.00	0.00	0.13	2.6
3-11	918.	1.50	3.50	633	374	0.00	0.47	0.00	0.00	0.47	6.0
3-10	-710.	1.50	3.50	633	363	0.00	0.37	0.00	0.00	0.37	4.9
4-10	1101.	1.50	3.50	633	374	0.00	0.56	0.00	0.00	0.56	7.2
4-9	1101.	1.50	3.50	633	374	0.00	0.56	0.00	0.00	0.56	7.2
5-9	-710.	1.50	3.50	633	363	0.00	0.37	0.00	0.00	0.37	4.9
5-8	918.	1.50	3.50	633	374	0.00	0.47	0.00	0.00	0.47	6.0
6-8	-337.	1.50	3.50	633	479	0.00	0.13	0.00	0.00	0.13	2.6

DEFLECTION AT 9 = -0.4069 INCHES
DEFLECTION BETWEEN 9-10 = -0.5036 INCHES

EXPLANATIONS:
P/AF = AXIAL FORCE DIVIDED BY THE CROSS-SECTIONAL AREA AND THE ALLOWABLE UNIT STRESS IN TENSION OR COMPRESSION.
VM/ZF= VERTICAL BENDING MOMENT DIVIDED BY THE SECTION MODULUS AND THE ALLOWABLE UNIT STRESS IN BENDING.
HM/ZF= HORIZONTAL BENDING MOMENT DIVIDED BY THE SECTION MODULUS AND THE ALLOWABLE UNIT STRESS IN BENDING.
CSI = COMBINED STRESS INTERACTION EQUATION IS THE ADDITION OF P/AF + VM/ZF + HM/ZF
LAT. = MAXIMUM DISTANCE ALLOWABLE (FT) WITHOUT ROUNDING OFF THE STRESSES SHOWN (FB FT FCI) ARE ALLOWABLE LUMBER STRESSES MODIFIED BY THE SHORT TERM INCREASE AND OTHER APPROPRIATE FACTORS WHEREVER APPLICABLE.

CHORDS SIZE LUMBER DESCRIPTION
1-4 2X 4 NO.2 KD15 SO. PINE
4-7 2X 4 NO.2 KD15 SO. PINE
7-1 2X 4 NO.1 D.KD15 SO.PINE

ALL WEBS 2X 4 NO.3 S.P.F.



MEMBER	FORCE (LBS)	CHORDS	SIZE	LUMBER DESCRIPTION	DESIGN CRITERIA
1 HL01 GNQ20 3.0X 5.0	1628C	1-3	2X 4	NO.2 KD15 SO. PINE	TOP CH. LL= 25 PSF
2 IN11 GNQ20 1.0X 4.0	1483C	3-5	2X 4	NO.2 KD15 SO. PINE	DL= 10 PSF
3 PR12 GNQ20 4.0X 4.0	1483C	5-1	2X 4	NO.2 KD15 SO. PINE	BOT CH. LL= 20 PSF
4 IN11 GNQ20 1.0X 4.0	1483C	5-1	2X 4	NO.2 KD15 SO. PINE	DL= 10 PSF
5 HL01 GNQ20 3.0X 5.0	1483C	5-1	2X 4	NO.2 KD15 SO. PINE	TOTAL LOAD= 65 PSF
6 IN02 GNQ20 3.0X 4.0	1406T				
7 IN02 GNQ20 3.0X 4.0	1406T				

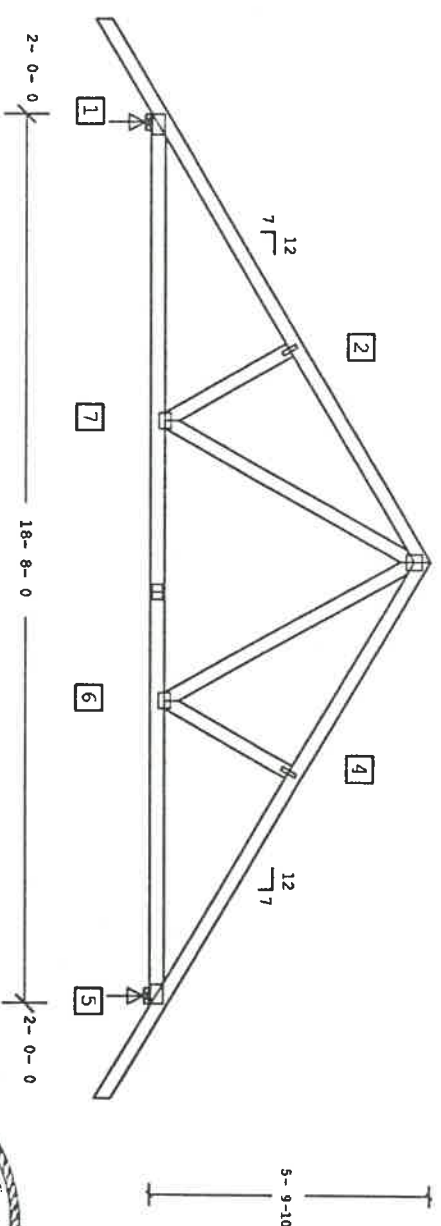
ALL WEBS 2X 4
 NO. 3 S.P.F.
 THIS TRUSS IS DESIGNED TO SUPPORT VERTICAL LOADS AS DETERMINED BY OTHERS AND SHOWN ON INPUT LISTING. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND BRACING OR OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED, IS THE RESPONSIBILITY OF THE PROJECT ARCHITECT OR ENGINEER.

MAX. PURLIN SPACE= 0.0 FT. * MAX. UNBRACED BOT. CH. LEN.= 10.0 FT.
 *** PLYWOOD SHEATHING REQUIRED ON TOP CHORD ***

NOTE: LATERAL BRACES AND PURLINS INDICATED FOR TRUSS MEMBERS ARE REQUIRED TO REDUCE BUCKLING LENGTH OF MEMBER, AND SHOULD BE NAILED TO TRUSS MEMBERS WITH MINIMUM OF 2-10D COMMON WIRE NAILS. PROVISIONS MUST BE MADE AT ENDS OR SPECIFIED INTERVALS TO RESTRAIN OR ANCHOR LATERAL BRACING. BY OTHERS.

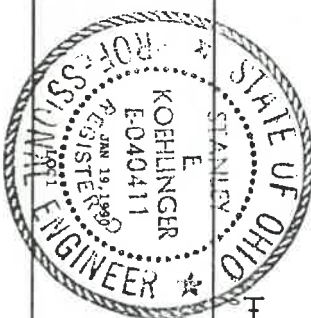
DESIGN SPEED. FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES, TPI, 1985

9D-5005-3
Rursed



Handling & Erection	Miscellaneous Information	Bracing Information	Connector Hardware	Lumber
Careful handling of components shall not be permitted. From plants and for resisting lateral loads shall be designed and installed by others. No loads are to be applied to the component until after all bracing and fastenings are complete. The use of this component shall be specified by the designer of the complete structure before using this component. Care must be exercised to install component at proper bearing points, right side up, and properly braced. Read all Gang Nail Systems, Inc. literature no control and accept no responsibility for the fabrication, handling, shipment and installation of components.	This data sheet and the information herein is the property of Gang Nail Systems, Inc. and is not to be copied in whole or in part or used for any other purpose without the written consent of Gang Nail Systems, Inc. The use of this component shall be specified by the designer of the complete structure before using this component. Care must be exercised to install component at proper bearing points, right side up, and properly braced. Read all Gang Nail Systems, Inc. literature no control and accept no responsibility for the fabrication, handling, shipment and installation of components.	All lateral bracing specified is for bracing members who members and web bracing where required are to be equally spaced along web length. Gang members are assumed to be braced by the steeling. Restraints of lateral bracing and pattern or casting materials. Lateral bracing is to be provided by the designer of the complete structure.	Connector plates are manufactured in accordance with TPI. Plates must be of the size, gauge and capacity shown. The AwoTing bolt detail shown for details of joint types and position of plates. Position plates and fasteners are shown.	Lumber must bear a grade mark from an approved source. Lumber shall be of the size and species shown and equal to or better than the grade specified.

IMPORTANT: READ ALL NOTES ON THIS DRAWING!



44A
0.31
0.18
0.35

44A	GROSS BRG	REACT	IN-SX
JT	1	1353	4-0
5	1353	4-0	

CAMBER= 0-0/8 HEEL= 4/16

11
JAN 19, 1990 F3-19F

WARNING- VERIFY YOUR INPUT TO AVOID DESIGN AND FABRICATION MISTAKES.
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REQUEST NO. CALCONLY QUOTE WAUSEON WOODWORKING (545624) TG
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13 7
REQUEST NO. CALCONLY QUOTE WAUSEON WOODWORKING (545624) TG T-4
JAN 19, 1990 F3-19F

SPAN (OUT TO OUT) 18.667
NO. OF JOINTS 7
LOC. OF REACTIONS 1 5
SHORT TERM INCREASE TCH 1.15 BCH 1.15 WEBS 1.15 PLATES 1.15
TOP CHORD LOAD SHARE = 15 BOTTOM CHORD LOAD SHARE = 15
PLYWOOD SHEATHING USED ON TOP CHORD WHERE APPLICABLE.

TRUSS GEOMETRY AND LOAD DATA	VER. DISP.	VERTICAL UNIF. LD.	VERTICAL CONC. LD.
JT HOR. DISP.	FT	SLOPE/12	PLF
1	4.8988	7.0000	-70.00
2	4.4345	7.0000	-70.00
3	4.4345	-7.0000	-70.00
4	4.8988	-7.0000	-70.00
5	-6.3770	0.0000	-60.00
6	-5.9127	0.0000	-60.00
7	-6.3770	0.0000	-60.00

TOTAL POSITIVE DISPLACEMENT= 18.67

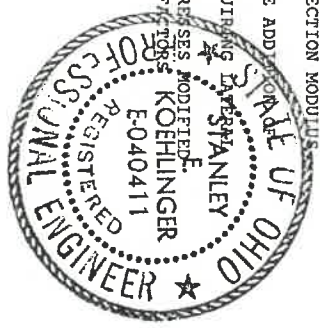
NO. OF WEBS= 4
2- 7 3- 7 3- 6 4- 6
GROSS REACTIONS(LBS):
RV- 1= 1353.3 RV- 5= 1353.3
RH- 1= 0.0

MEM	FORCE	WTD	DEP	FB	FC:FT	Q	P/AF	VM/ZF	HM/ZF	CSI	LAT.
	LBS	IN.	IN.	PSI	PSI					BRC	
TOP CHORD MEMBERS											
1- 2	-1628.	1.50	3.50	2013	1323	0.90	0.23	0.27	0.00	0.51	0.0
2- 3	-1483.	1.50	3.50	2013	1323	0.90	0.21	0.27	0.00	0.48	0.0
3- 4	-1483.	1.50	3.50	2013	1323	0.90	0.21	0.27	0.00	0.48	0.0
4- 5	-1628.	1.50	3.50	2013	1323	0.90	0.23	0.27	0.00	0.51	0.0
BOT CHORD MEMBERS											
5- 6	1406.	1.50	3.50	2013	1035	1.00	0.26	0.52	0.00	0.78	10.0
6- 7	986.	1.50	3.50	2013	1035	1.00	0.18	0.52	0.00	0.70	10.0
7- 1	1406.	1.50	3.50	2013	1035	1.00	0.26	0.52	0.00	0.78	10.0
WEB MEMBERS											
2- 7	-272.	1.50	3.50	633	465	0.00	0.11	0.00	0.00	0.11	3.2
3- 7	880.	1.50	3.50	633	374	0.00	0.35	0.00	0.00	0.35	6.2

CHORDS	SIZE	LUMBER DESCRIPTION
1- 3	2X 4	NO.2 KD15 SO. PINE
3- 5	2X 4	NO.2 KD15 SO. PINE
5- 1	2X 4	NO.2 KD15 SO. PINE

ALL WEBS 2X 4 NO.3 S.P.F.

DEFLECTION AT 6 = -0.0867 INCHES
DEFLECTION BETWEEN 6- 7 = -0.1944 INCHES
EXPLANATIONS:
P/AF = AXIAL FORCE DIVIDED BY THE CROSS-SECTIONAL AREA AND THE ALLOWABLE UNIT STRESS IN TENSION OR COMPRESSION.
VM/ZF= VERTICAL BENDING MOMENT DIVIDED BY THE SECTION MODULUS AND THE ALLOWABLE UNIT STRESS IN BENDING.
HM/ZF= HORIZONTAL BENDING MOMENT DIVIDED BY THE SECTION MODULUS AND THE ALLOWABLE UNIT STRESS IN BENDING.
CSI = COMBINED STRESS INTERACTION EQUATION IS THE ADDITIONAL FACTOR.
P/AF + VM/ZF + HM/ZF
LAT. = MAXIMUM DISTANCE ALLOWABLE (FT) WITHOUT REQUIRING SUPPORT.
BRC = BENDING RESISTANCE COEFFICIENT.
STRESSES SHOWN (FB FT FC) ARE ALLOWABLE LUMBER STRESSES MODIFIED BY THE SHORT TERM INCREASE AND OTHER APPROPRIATE FACTORS WHEREVER APPLICABLE.



MEMBER	FORCE (LBS)	CHORDS	SIZE	LUMBER DESCRIPTION	DESIGN CRITERIA
1 H1.01 GNO20 3.0X 8.0	4113C	1-4	2X 4	NO.1 KD15 SO. PINE	TOP CH. LL= 25 PSF
2 H1.11 GNO20 1.0X 4.0	3919C	4-6	2X 4	NO.1 KD15 SO. PINE	DL= 10 PSF
3 H1.02 GNO20 3.0X 5.0	2575C	6-1	2X 4	NO.1 D.KD15 SO. PINE	BOT CH. LL= 20 PSF
4 H1.12 GNO20 4.0X 5.0	2481C	6-1	2X 4	NO.1 D.KD15 SO. PINE	DL= 10 PSF
5 H1.11 GNO20 1.0X 4.0	2481C	4-5	2X 4	NO.1 D.KD15 SO. PINE	TOTAL LOAD= 65 PSF
6 H1.01 GNO20 3.0X 6.0	2628C	4-11	2X 4	NO.3 S.P.F.	
7 H1.02 GNO20 3.0X 4.0	2370T	6-4	2X 4	NO.3 S.P.F.	
8 H1.02 GNO20 3.0X 5.0	1809T	7-8	2X 4	NO.3 S.P.F.	
9 H1.02 GNO20 3.0X 4.0	2865T	8-9	2X 4	NO.3 S.P.F.	
10 H1.02 GNO20 3.0X 4.0	3919T	9-1	2X 4	NO.3 S.P.F.	

NOTE: LATERAL BRACES AND PURLINS INDICATED FOR TRUSS MEMBERS ARE REQUIRED TO REDUCE BUCKLING LENGTH OF MEMBER, AND SHOULD BE MAILED TO TRUSS MEMBERS WITH MINIMUM OF 2-10D COMMON WIRE NAILS. PROVISIONS MUST BE MADE AT ENDS OR SPECIFIED INTERVALS TO RESTRAIN OR ANCHOR LATERAL BRACING, BY OTHERS.

THIS TRUSS IS DESIGNED TO SUPPORT VERTICAL LOADS AS DETERMINED BY OTHERS AND SHOWN ON INPUT LISTING. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND BRACING OR OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED, IS THE RESPONSIBILITY OF THE PROJECT ARCHITECT OR ENGINEER.

DESIGN CRITERIA
 TOP CH. LL= 25 PSF
 DL= 10 PSF
 BOT CH. LL= 20 PSF
 DL= 10 PSF
 TOTAL LOAD= 65 PSF

DESIGN DEFLECTION= 24 IN. C/C

INPUT DEFLECTION= 1.7360

INCREASES (PER CENT)
 LUMBER= 15
 NAIL= 15
 TOP LS= 15
 BCH LS= 15

MAX. PURLIN SPACING= 0.0 FT., MAX. UNBRACED BOT. CH. LEN.= 10.0 FT.
 *** PLWOOD SHEATHING REQUIRED ON TOP CHORD ***

MAXIMUM MEMBER FORCE (LBS)
 MEMBER FR-TO
 1-2 4113C
 2-3 3919C
 3-4 2575C
 4-5 2481C
 5-6 2628C
 6-7 2370T
 7-8 1809T
 8-9 2865T
 9-1 3919T

MAXIMUM UNBRACED MEMBER LENGTH (PLF)
 MEMBER FR-TO
 1-2 70.0
 2-3 70.0
 3-4 70.0
 4-5 70.0
 5-6 70.0
 6-7 70.0
 7-8 60.0
 8-9 60.0
 9-1 60.0

MAXIMUM MEMBER FORCE (LBS)
 MEMBER FR-TO
 2-9 387C
 3-9 1145T
 3-8 819C
 4-8 1258T
 4-7 700T
 5-7 276C

CHORDS
 1-4
 4-6
 6-1

SIZE
 2X 4
 2X 4
 2X 4

LUMBER DESCRIPTION
 NO.1 KD15 SO. PINE
 NO.1 KD15 SO. PINE
 NO.1 D.KD15 SO. PINE

DESIGN CRITERIA
 TOP CH. LL= 25 PSF
 DL= 10 PSF
 BOT CH. LL= 20 PSF
 DL= 10 PSF
 TOTAL LOAD= 65 PSF

DESIGN DEFLECTION= 24 IN. C/C

INPUT DEFLECTION= 1.7360

INCREASES (PER CENT)
 LUMBER= 15
 NAIL= 15
 TOP LS= 15
 BCH LS= 15

MAX. PURLIN SPACING= 0.0 FT., MAX. UNBRACED BOT. CH. LEN.= 10.0 FT.
 *** PLWOOD SHEATHING REQUIRED ON TOP CHORD ***

MAXIMUM MEMBER FORCE (LBS)
 MEMBER FR-TO
 1-2 4113C
 2-3 3919C
 3-4 2575C
 4-5 2481C
 5-6 2628C
 6-7 2370T
 7-8 1809T
 8-9 2865T
 9-1 3919T

MAXIMUM UNBRACED MEMBER LENGTH (PLF)
 MEMBER FR-TO
 1-2 70.0
 2-3 70.0
 3-4 70.0
 4-5 70.0
 5-6 70.0
 6-7 70.0
 7-8 60.0
 8-9 60.0
 9-1 60.0

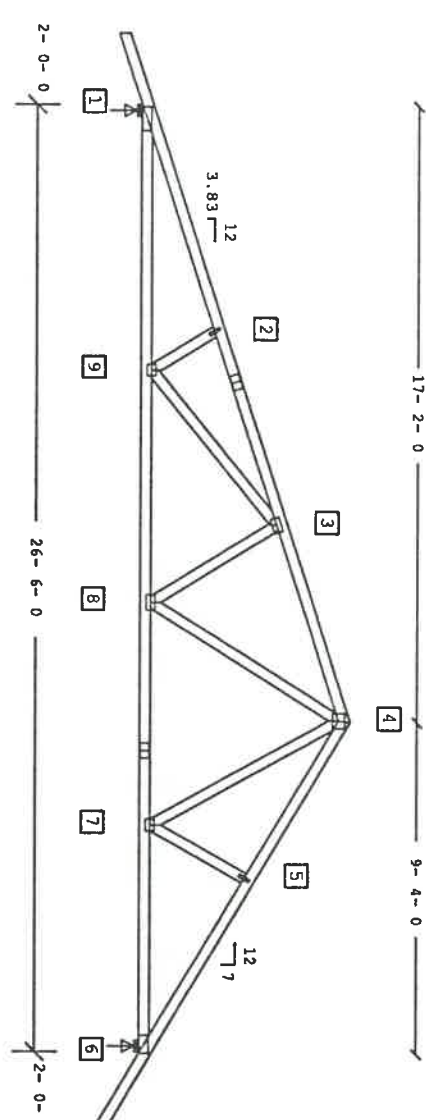
MAXIMUM MEMBER FORCE (LBS)
 MEMBER FR-TO
 2-9 387C
 3-9 1145T
 3-8 819C
 4-8 1258T
 4-7 700T
 5-7 276C

CHORDS
 1-4
 4-6
 6-1

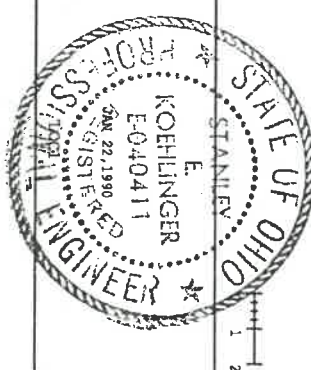
SIZE
 2X 4
 2X 4
 2X 4

LUMBER DESCRIPTION
 NO.1 KD15 SO. PINE
 NO.1 KD15 SO. PINE
 NO.1 D.KD15 SO. PINE

90-5005-4
 Revised



Handling & Erection	Miscellaneous Information	Bracing Information	Connector Hardware	Lumber
Caution handling of components must be observed. Temporary and permanent bracing for both components and joints must be provided and maintained until the truss is fully braced and ready for erection. No loads are to be applied to the truss until it is fully braced and ready for erection. All no man shall loads greater than design loads be applied to the component.	This data sheet and the information herein is the property of Gang-Nail Systems, Inc. It is to be used for the design and construction of the truss only. It is not to be used for any other purpose. The designer of the complete structure is to be responsible for the design and construction of the complete structure. When the drawing is signed and sealed, Gang-Nail Systems, Inc. is not responsible for the design and construction of the complete structure.	All lateral bracing specified is for bracing of the truss members and web bracing where required. Web bracing where required is to be equally spaced along web length. Lateral bracing of truss members is to be provided by the designer of the complete structure.	Connector plates are manufactured in accordance with the specifications of the American Institute of Steel Construction, Inc. (AISC) and are to be used in accordance with the specifications of the American Institute of Steel Construction, Inc. (AISC) and are to be used in accordance with the specifications of the American Institute of Steel Construction, Inc. (AISC).	Lumber must be a grade mark from an approved manufacturer and be of the size and species shown and equal to or better than the grade specified.



IMPORTANT: READ ALL NOTES ON THIS DRAWING!

90/01/22
15:42:59

1289q-545624.ans

6

11
JAN 22, 1990 F3-19F

WARNING- VERIFY YOUR INPUT TO AVOID DESIGN AND FABRICATION MISTAKES.
YOU ARE SOLELY RESPONSIBLE FOR ERRORS RESULTING FROM WRONG INPUT

REQUEST NO. CALCONLY QUOTE WAUSEON WOODWORKING (545624) TG

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13 7
REQUEST NO. CALCONLY QUOTE WAUSEON WOODWORKING (545624) TG T-5
JAN 22, 1990 F3-19F

SPAN (OUT TO OUT) 26.500
NO. OF JOINTS 9
LOC. OF REACTIONS 1 6
SHORT TERM INCREASE TCH 1.15 BCH 1.15 WEBS 1.15 PLATES 1.15

TOP CHORD LOAD SHARE = 15 BOTTOM CHORD LOAD SHARE = 15
PLYWOOD SHEATHING USED ON TOP CHORD WHERE APPLICABLE.

TRUSS GEOMETRY AND LOAD DATA

JT	HOR. DISP. FT	VER. DISP. SLOPE/12	VERTICAL UNIF. ID.	VERTICAL CONC. ID.
1	6.2768	3.8275	-70.00	-140.00
2	5.4277	3.8275	-70.00	0.00
3	5.4277	3.8275	-70.00	0.00
4	4.4517	-7.0000	-70.00	0.00
5	4.9160	-7.0000	-70.00	0.00
6	-6.3999	0.0000	-60.00	-140.00
7	-6.2244	0.0000	-60.00	0.00
8	-6.5133	0.0000	-60.00	0.00
9	-7.3624	0.0000	-60.00	0.00

TOTAL POSITIVE DISPLACEMENT= 26.50

NO. OF WEBS= 6
2- 9 3- 9 3- 8 4- 8 4- 7 5- 7

GROSS REACTIONS (LBS):
RV- 1= 1862.5 RV- 6= 1862.5
RH- 1= 0.0

MEM FORCE MID DEP FB FC:FT Q P/AF VM/2F HM/2F CSI LAT. IN. IN. PSI PSI PSI

TOP CHORD MEMBERS

1- 2	-4113.	1.50	3.50	2091	1542	0.80	0.51	0.33	0.00	0.84	0.0
2- 3	-3919.	1.50	3.50	2106	1542	0.80	0.48	0.33	0.00	0.81	0.0
3- 4	-2575.	1.50	3.50	2231	1556	0.80	0.32	0.29	0.00	0.60	0.0
4- 5	-2481.	1.50	3.50	2415	1668	0.90	0.28	0.23	0.00	0.51	0.0
5- 6	-2628.	1.50	3.50	2415	1668	0.90	0.30	0.23	0.00	0.53	0.0

BOF CHORD MEMBERS

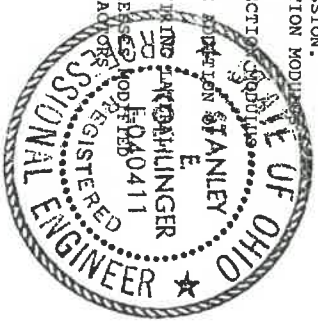
6- 7	2270.	1.50	3.50	2818	1438	1.00	0.30	0.39	0.00	0.69	10.0
7- 8	1809.	1.50	3.50	2818	1438	1.00	0.24	0.42	0.00	0.66	10.0
8- 9	2865.	1.50	3.50	2818	1438	1.00	0.38	0.48	0.00	0.86	10.0

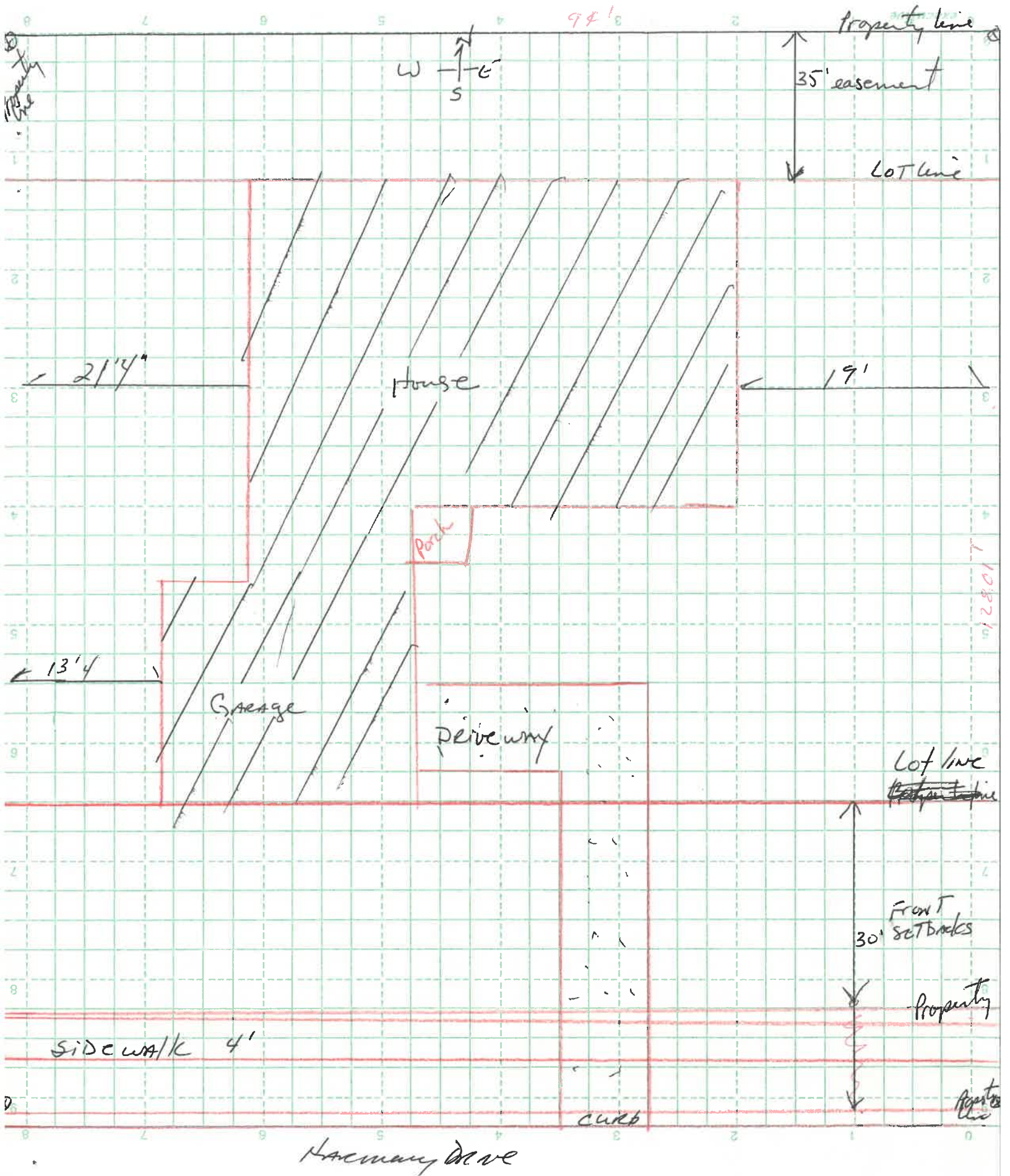
MEMBERS	9- 1	2- 9	3- 9	4- 8	4- 7	5- 7
3918.	1.50	3.50	2618	1438	1.00	0.52
-387.	1.50	3.50	633	483	0.00	0.15
1145.	1.50	3.50	633	374	0.00	0.58
-819.	1.50	3.50	633	411	0.00	0.38
1258.	1.50	3.50	633	374	0.00	0.00
700.	1.50	3.50	633	374	0.00	0.36
-276.	1.50	3.50	633	464	0.00	0.11

DEFLECTION AT 9 = -0.3435 INCHES
DEFLECTION BETWEEN 8- 9 = -0.4567 INCHES

EXPLANATIONS:
P/AF = AXIAL FORCE DIVIDED BY THE CROSS-SECTIONAL AREA AND THE ALLOWABLE UNIT STRESS IN TENSION OR COMPRESSION.
VM/2F = VERTICAL BENDING MOMENT DIVIDED BY THE SECTION MODULUS AND THE ALLOWABLE UNIT STRESS IN BENDING.
HM/2F = HORIZONTAL BENDING MOMENT DIVIDED BY THE SECTION MODULUS AND THE ALLOWABLE UNIT STRESS IN BENDING.
CSI = COMBINED STRESS INTERACTION EQUATION IS THE ALLOWABLE STRESS SUPPORT.
P/AF + VM/2F + HM/2F
LAT. = MAXIMUM DISTANCE ALLOWABLE (FT) WITHOUT REQUIREMENT FOR STRESS SUPPORT.
STRESSES SHOWN (FB FT FC) ARE ALLOWABLE LUMBER STRESSES MODIFIED BY THE SHORT TERM INCREASE AND OTHER APPROPRIATE FACTORS WHEREVER APPLICABLE.

CHORDS	SIZE	LUMBER DESCRIPTION
1- 4	2X 4	NO.1 KD15 SO. PINE
4- 6	2X 4	NO.1 KD15 SO. PINE
6- 1	2X 4	NO.1 D.KD15 SO.PINE
ALL WEBS	2X 4	NO.3 S.P.F.



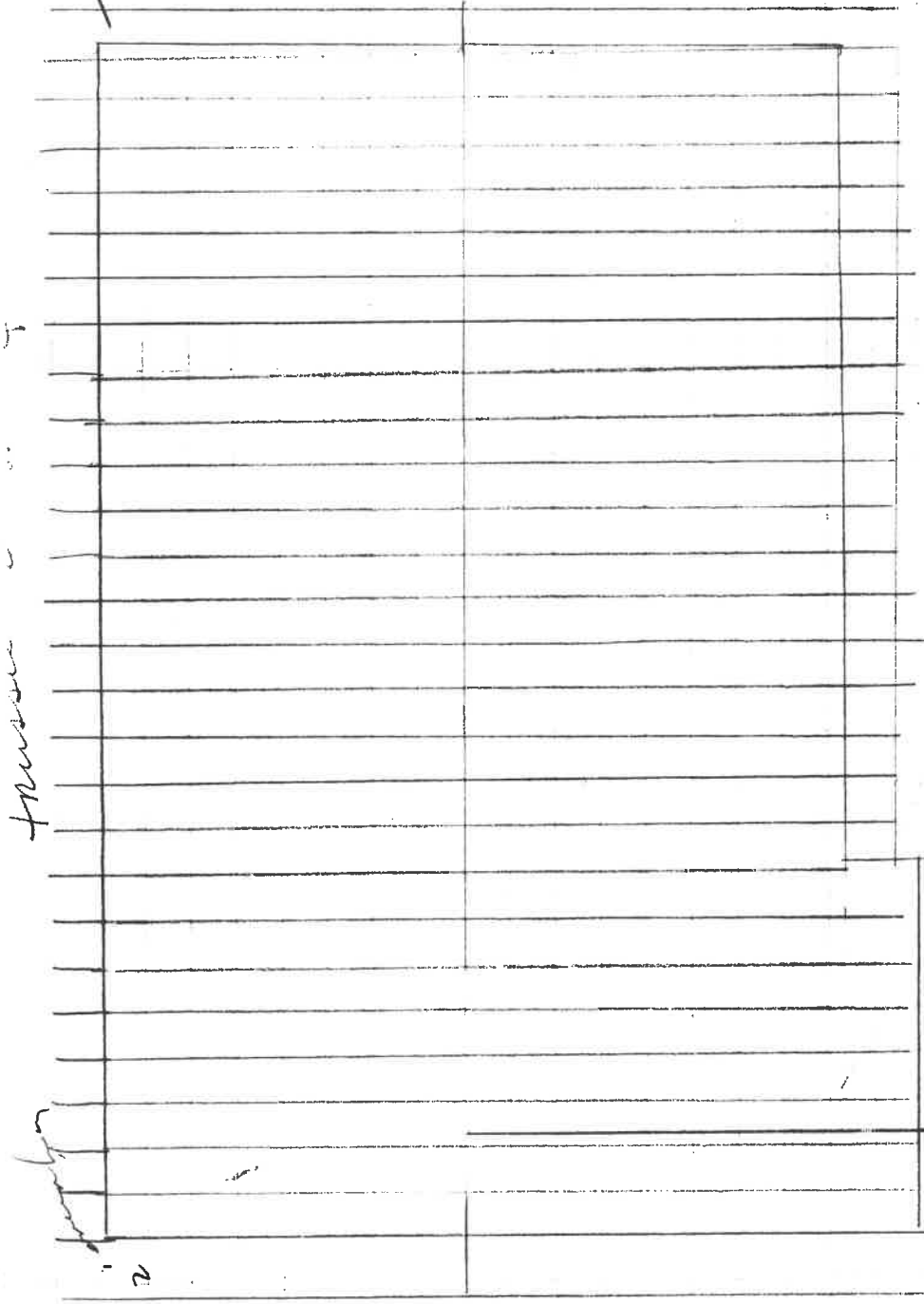


DATE	
PREPARED BY	

 Plot plan

PAGE NO.	

2' overhang



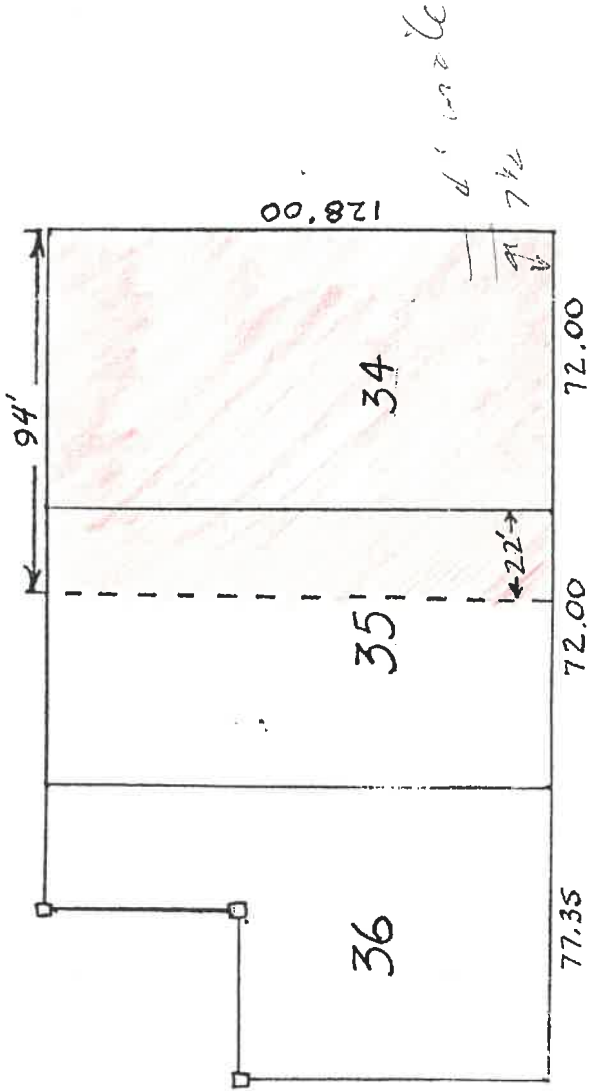
trusses

Gambrel trusses



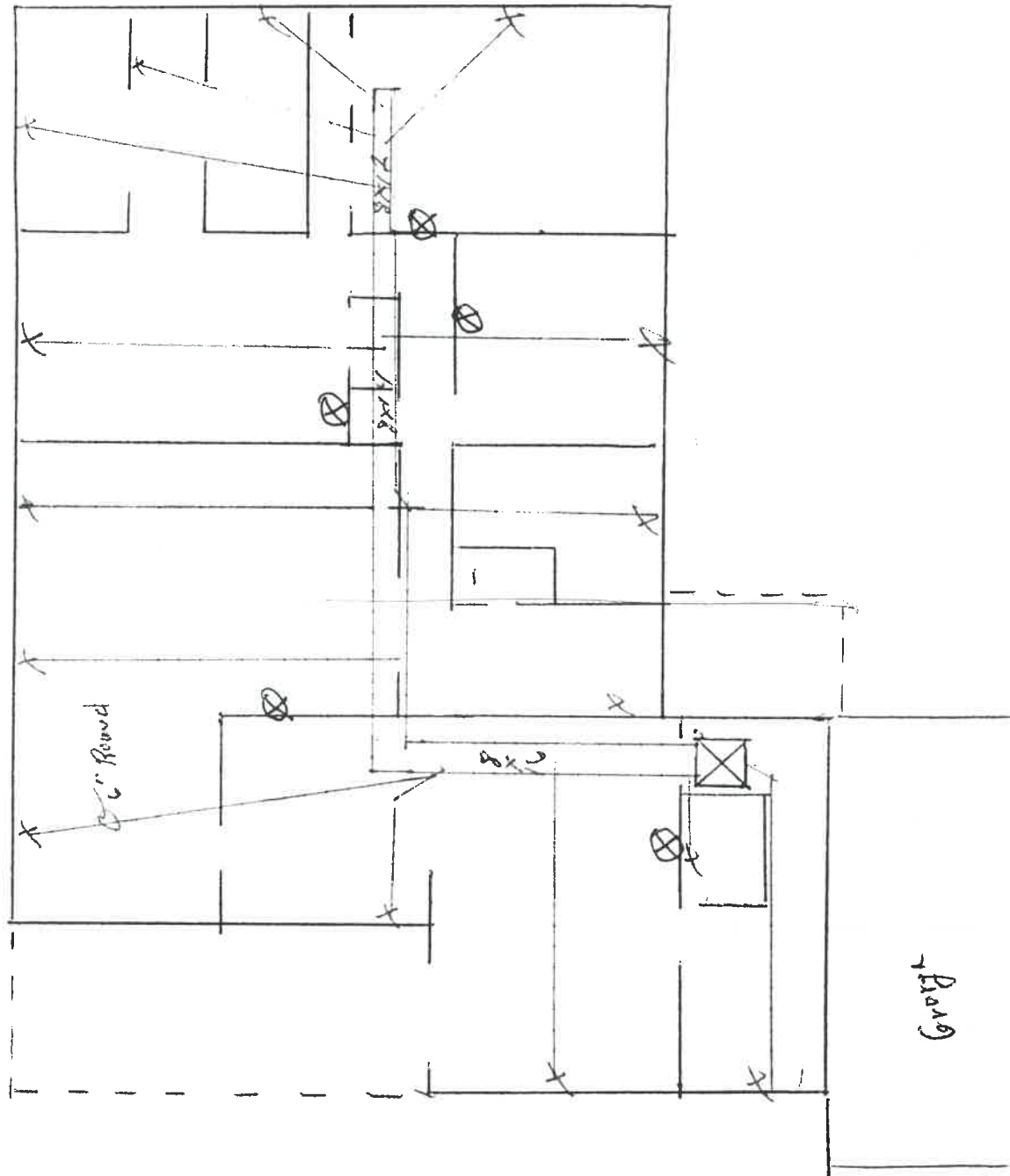
roof

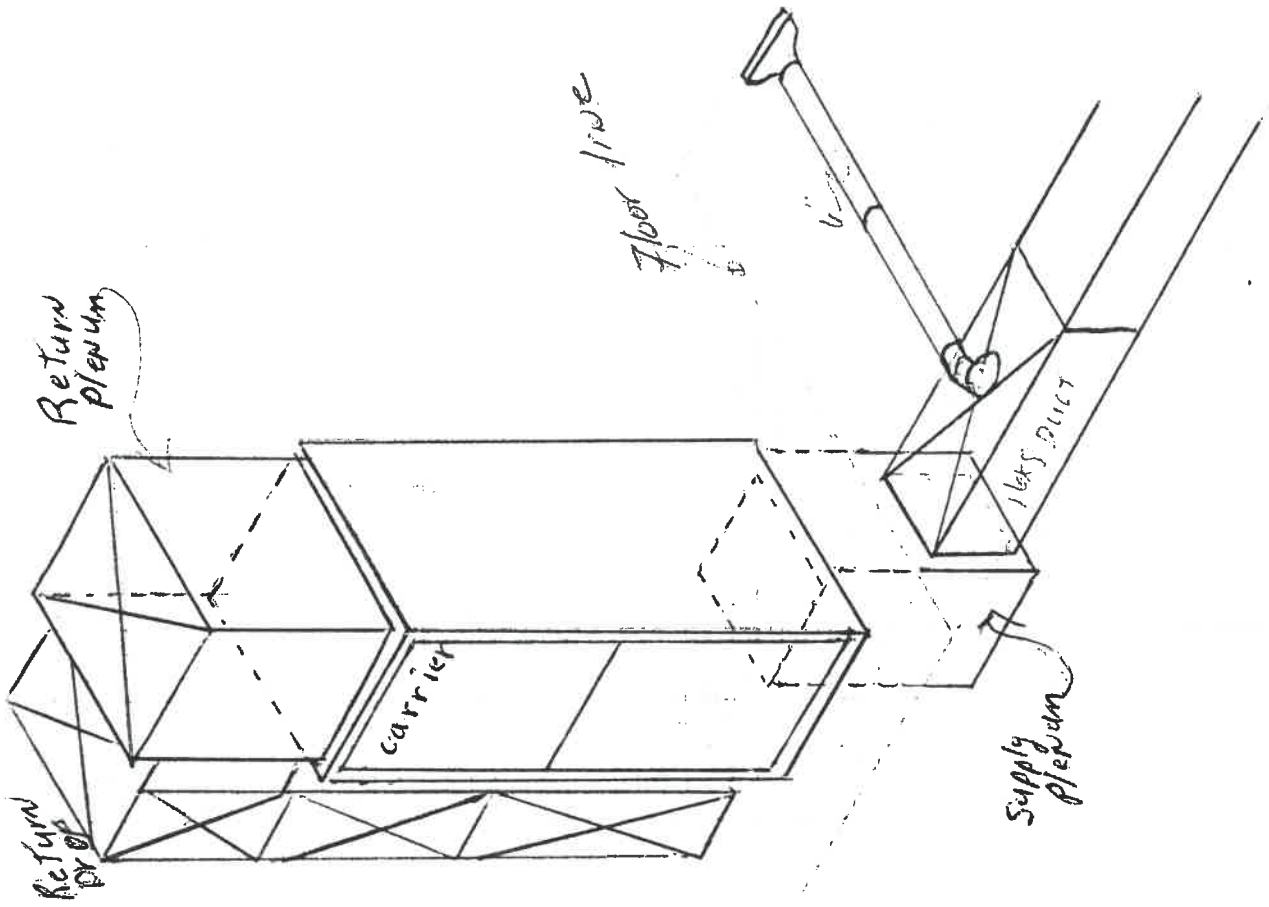
BRICKYARD SUBDIVISION



SIDE L.A.H. 7'21'
4' 10' 10''

⊗ Return Air
X Supply Air





Return and Supply
Plenum To Be of

26 GA Metal

Supply and Return
Ducting To Be of

28 GA Metal

Round Ducting To

30 Mill Wire gound

Plastic with 1/2" vinyl

coverd Fiberglass

Per 2607

Supply Metal Ducting

To Be Insulated with

1" Fiberglass with 1/2" vinyl

Facing Ductwrap

